SHEETS NOT INCLUDED IN TOTAL SHEETS

STANDARD DRAWINGS

DESIGN CRITERIA

GEOGRAPHIC COORDINATES LATITUDE 37 DEGREES 20 MINUTES 46 SECONDS NORTH

LONGITUDE 82 DEGREES 51 MINUTES 29 SECONDS WEST

DESIGNED

CLASS OF HIGHWAY RURAL LOCAL ROAD TYPE OF TERRAIN ROLLING DESIGN SPEED

REQUIRED NPSD

LEVEL OF SERVICE -ADT PRESENT (-) ___ ADT FUTURE (-) ___

% RESTRICTED SD _

LEVEL OF SERVICE _ MAX. DISTANCE W/O PASSING

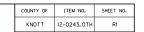
REQUIRED PSD

Commonwealtl	h of	Ken	tucky
DEPARTMENT	OF	HIGH	IWAY

PLANS OF PROPOSED PROJECT

KNOTT COUNTY GRADE, DRAIN & SURFACE WITH STRUCTURE PLANS HOLLYBUSH ROAD CR 1108 OVER HOLLYBUSH CREEK

060C00022N





RBI-001-12 RBI-002-07 RBR-001-13 RBR-005-11 RBR-010-06 RBR-015-06 RBR-016-05 RBR-050-08 RBR-051-01 RDD-040-05 RD1-004-04 RDX-210-03 RDX-220-05 RDX-225-01 RGX-001-06 RGX-100-07 RGX-105-09 RGX-200-01 TTC-150-04 TTC-155-02

STA. 11+43.50 CONST. BRIDGE OVER HOLLYBUSH CREEK STA. 12+87.66 END CONST. STA. 10+00.00 BEGIN CONST.

THIS PROJECT IS OFF THE NH SYSTEM

THE CONTROL OF ACCESS ON THIS PROJECT SHALL BE BY PERMIT

LAYOUT MAP

LENGTH.

HOLLYBUSH ROAD

LIN. FT. RAILROAD CROSSINGS NO. _ LIN. FT. BRIDGES RAILROAD CROSSINGS NO. __

MILES LENGTH LIN. FT. ADDED FOR EQUALITIES NOT INCLUDED ___LIN. FT. ___ __LIN. FT. ADDED FOR EQUALITIES ________
NOT INCLUDED _LIN. F1 _ LIN. FT. RAILROAD CROSSINGS NO. __ _LIN. FT. BRIDGES ____ LIN. FT. RAILROAD CROSSINGS NO. LIN. FT. BRIDGES

Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS COUNTY OF

KNOTT

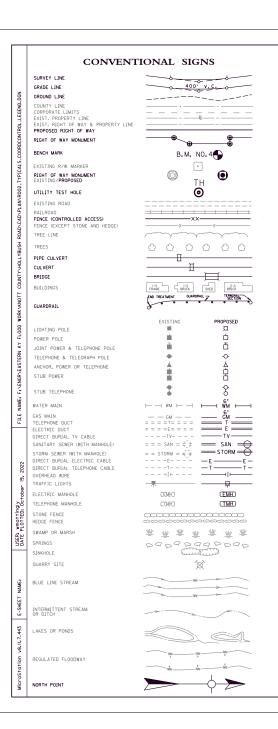
ITEM NO. 12-0243.0TH PROJECT 060C00022N D23A 4663-DR LETTING DATE:

STATE HIGHWAY ENGINEER

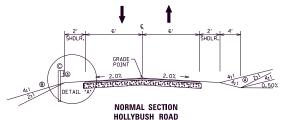


3 HMB Circle Frankfort, KY 4060 (502) 695-9801 (502) 695-9810 FAX



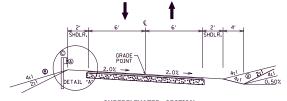


TYPICAL SECTIONS



- B See Cross Sections for Slopes Outside the Limits of the Shoulders.
- © When guardrail is used, pave shoulder to face of rail.

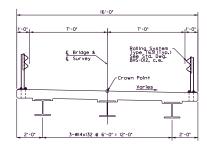
TYPICAL SECTIONS



SUPERELEVATED SECTION HOLLYBUSH ROAD

- B See Cross Sections for Slopes Outside the
- © When guardrall is used, pave shoulder to face of rail. TRAFFIC LANE & SHOULDER PAVEMENT

DGA Base 3" Depth CL. 2 Asphalt Base 1.00D PG64-22 Asphalt Surface 11/2" Depth Cl. 2 Asphalt Surface 0.38D PG64-22



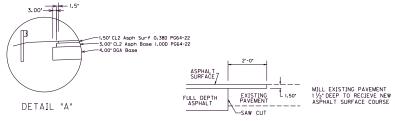
NOTE: Roadway Guardrail is Attached to Bridge Barriers, See Roadway Plans,

BRIDGE TYPICAL SECTION

COUNTY OF	ITEM NO.	SHEET NO.
KNOTT	12-0243.0TH	R2

	COORDINATE CONTROL POINTS											
POINT	DESCRIPTION	Stat	e Plane Coordina	STATION	OFFSET							
POINT	DESCRIPTION	NORTH (Y)	EAST (X)	ELEV. (Z)	STATION	OFFSET						
CP 1	PK NAIL/WASHER	3662687.2010	5761879.8540	894.142	12+55.06	-5.42						
CP 2	PK NAIL/WASHER	3662531.5140	5761664.7560	895.295	9+88.13	-4.65						
CP 3	PK NAIL/WASHER	3662530.3520	5761686.4210	896.044	10+04.15	9.98						

	CENTERLINE CONTROL POINTS											
POINT	State Plane	Coordinates	BADILIS	LENGTH	TANGENT	STATION	OFFSET					
	NORTHING (Y)	EASTING (X)	IVADIOS	LLIVOIII	IANGLINI	SIATION	OTTSET					
POB	3662535.4395	5761676.8865				10+00.00	0.00					
PC	3662514.0233	5761687.3746				10+13.55	0.00					
PI	3662559.7755	5761706.6213	500	49.7	24.87	10+38.42	-0.78					
PT	3662573.5400	5761727.3362				10+63.25	0.00					
PC	3662533.3479	5761742.0969				10+80.98	0.00					
PI	3662595.6549	5761760.6184	154	44.17	22.24	11+03.21	1.60					
PT	3662612.6990	5761774.9017				11+25.15	0.00					
PC	3662625.1264	5761785.3162				11+41.36	0.00					
PI	3662656.9945	5761812.0223	154	81.22	41.58	11+82.94	-5.51					
PT	3662671.0910	5761851.1385				12+22.58	0.00					
POE	3662693.1555	5761912.3645				12+87.66	0.00					



EDGE KEY DETAIL

UTILITY OWNERS

ELECTRIC SERVICES: KY Power Company 1400 East Main St Hazard, KY 41701

Contact Ellis McKnight 606-436-1329

TV SERVICES: TV Services

60 Communications Lane P.O. Box 789 Hindman, KY 41822

Contact Freddy Williams 606-785-9500

TELEPHONE SERVICES:

Thacker Grigsby Telephone Co. 60 Communications Lane P.O. Box 789

Hindman, KY 41822

Contact Freddy Williams 606-785-9500

Datum Reference and Final Coordinates All new horizontal GNSS control is based on the Kentucky State Plane Coordinate System (Single Zone), referenced to North American Datum 1983, 2011 adjustment, expressed in U.S. Survey Feet. All vertical control is based on the North American Vertical Datum of 1988 (NAVD88) with GEOID18 (CONUS) applied to model the elevations, also expressed in U.S. Survey Feet.

TYPICAL SECTIONS, COORD, CONTROL, AND LEGEND

SCALE: N.T.S

GENERAL SUMMARY

00001 DGA BASE ① TON 00020 TRAFFIC BOUND BASE ① TON 00078 CRUSHED AGGREGATE SIZE NO 2 ② ⑩ TON 00212 CL2 ASPH BASE 1.00D PG64-22 TON 00301 CL2 ASPH SURF 0.38D PG64-22 TON 02014 BARRICADE - TYPE III EA 02159 TEMP DITCH LF 02160 CLEAN TEMP DITCH LF 02200 ROADWAY EXCAVATION CUYD 02242 WATER ② MGAL 18002EN G/R STEEL W BEAM-S FACE (7 FT POST) LF 02360 GUARDRAIL TERMINAL SECTION NO 1 EACH 02371 GUARDRAIL END TREATMENT TYPE 7 EACH 02371 GUARDRAIL TERMINAL SECTION NO 1 EACH 02380 GUARDRAIL TERMINAL SECTION NO 1 EACH 02565 </th <th>ITEM</th> <th>DESCRIPTION</th> <th>UNIT</th> <th>PROJECT</th>	ITEM	DESCRIPTION	UNIT	PROJECT
00078 CRUSHED AGGREGATE SIZE NO 2 ③⑥ TON 00212 CL2 ASPH BASE 1.00D PG64-22 TON 00301 CL2 ASPH BASE 1.00D PG64-22 TON 02014 BARRICADE - TYPE III EA 02159 TEMP DITCH LF 02160 CLEAN TEMP DITCH LF 02200 ROADWAY EXCAVATION CUYD 02242 WATER ② MGAL 21802EN G/R STEEL W BEAM-S FACE (7 FT POST) LF 02360 GUARDRAIL TERMINAL SECTION NO 1 EACH 02371 GUARDRAIL END TREATMENT TYPE 7 EACH 02371 GUARDRAIL END TREATMENT TYPE 7 EACH 02371 GUARDRAIL END TREATMENT TYPE 7 EACH 02371 GUARDRAIL TERMINAL SECTION NO 1 EACH 02371 GUARDRAIL TERMINAL SECTION NO 1 EACH 02371 GUARDRAIL TERMINAL SECTION NO 1 EACH 02565 CLEARING AND GRUBBING ③ LS 025662 TEMPORARY SIGNS SOFT 025655 DEJECT MARKER TYPE 2 ⑧	00001	DGA BASE ①	TON	3
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00212 CL2 ASPH BASE 1.00D PG64-22 TON 00301 CL2 ASPH SURF 0.38D PG64-22 TON 02014 BARRICADE - TYPE III EA 02159 TEMP DITCH LF 02160 CLEAN TEMP DITCH LF 02200 ROADWAY EXCAVATION CUYD 02242 WATER ② MGAL 21802EN GR STEEL W BEAM-S FACE (7 FT POST) LF 02360 GUARDRAIL ERM TREATMENT TYPE 7 EACH 02371 GUARDRAIL END TREATMENT TYPE 7 EACH 02484 CHANNEL LINING CLASS II ⑥ TON 02545 CLEARING AND GRUBBING ③ LS 02565 OBJECT MARKER TYPE 2 ③ EACH 02565 OBJECT MARKER TYPE 2 ④ EACH 02566 DEMOBILIZATION LS 02669 DEMOBILIZATION LS 02661 DIVERSIONS (BY-PASS DETOURS) LS 026650 MAINTAIN & CONTROL TRAFFIC ④ LS 026671 PORTABLE CHANGEABLE MESSAGE SIGN EA 027726 STAKING LS	00078	CRUSHED AGGREGATE SIZE NO 2 9(0)	TON	
D2014 BARRICADE - TYPE III	00212		TON	2
D2014 BARRICADE - TYPE III	00301	CL2 ASPH SURF 0.38D PG64-22	TON	
02159 TEMP DITCH LF 02160 CLEAN TEMP DITCH LF 02200 ROADWAY EXCAVATION CUYD 02242 WATER ② MGAL 21802EN GIR STEEL W BEAM-S FACE (7 FT POST) LF 02360 GUARDRAIL ERMINAL SECTION NO 1 EACH 02371 GUARDRAIL END TREATMENT TYPE 7 EACH 02484 CHANNEL LINING CLASS III ⑤ TON 02545 CLEARING AND GRUBBING ③ LS 02562 TEMPORARY SIGNS SQFT 02565 OBJECT MARKER TYPE 2 ⑥ EACH 02565 DEMOBILIZATION LS 02566 DEMOBILIZATION LS 02603 FABRIC - GEOTEXTILE CLASS 2 ③ SQYD 02650 MAINTAIN & CONTROL TRAFFIC ﴿ LS 02651 DIVERSIONS (BY-PASS DETOURS) LS 02651 DIVERSIONS (BY-PASS DETOURS) LS 02726 STAKING LS 02731 REMOVE STRUCTURE LS 02726				
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02562 TEMPORARY SIGNS SQFT 02565 OBJECT MARKER TYPE 2 (8) EACH 02569 DEDGE KEY LF 02569 DEMOBILIZATION LS 02603 FABRIC - GEOTEXTILE CLASS 2 (9) SQYD 02650 MAINTAIN & CONTROL TRAFFIC (4) LS 02651 DIVERSIONS (BY-PASS DETOURS) LS 02671 PORTABLE CHANGEABLE MESSAGE SIGN EA 02726 STAKING LS 02731 REMOVE STRUCTURE LS 02701 TEMPORARY SILT FENCE LF 02703 SILT TRAP TYPE "A" EACH 02704 SILT TRAP TYPE "B" EACH 02706 CLEAN SILT TRAP TYPE "B" EACH 02707 CLEAN SILT TRAP TYPE "B" EACH 03171 CONCRETE BARRIER WALL TYPE 9T LF 04933 TEMP SIGNAL 2 PHASE (1) EA 04933 TEMP SIGNAL 2 PHASE (1) EA 05950 EROSION CONTROL BLANKET SQYD		<u>~</u>		<u> </u>
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02585 EDGE KEY LF 02569 DEMOBILIZATION LS 02603 FABRIC - GEOTEXTILE CLASS 2 ③ SQYD 02650 MAINTAIN & CONTROL TRAFFIC ④ LS 02651 DIVERSIONS (BY-PASS DETOURS) LS 02671 PORTABLE CHANGEABLE MESSAGE SIGN EA 02726 STAKING LS 02731 REMOVE STRUCTURE LS 02701 TEMPORARY SILT FENCE LF 02703 SILT TRAP TYPE "A" EACH 02704 SILT TRAP TYPE "B" EACH 02705 CLEAN SILT TRAP TYPE "B" EACH 02707 CLEAN SILT TRAP TYPE "B" EACH 02707 CLEAN SILT TRAP TYPE "B" EACH 03171 CONCRETE BARRIER WALL TYPE 9T LF 04933 TEMP SIGNAL 2 PHASE ① EA 05950 EROSION CONTROL BLANKET SQYD 05952 TEMP MULCH SQYD 05953 TEMP SEEDING AND PROTECTION SQYD 05964 MAINTENANCE FERTILIZER TON </td <td></td> <td></td> <td></td> <td>3</td>				3
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06587 PAVEMENT MARKER TY IVA-BW TEMP EA 06510 PAVE STRIPING - TEMP PAINT - 4 IN LF 08901 CRASH CUSHION TY VI CLASS BT TL2 EA 24970EC ASPHALT MATERIAL FOR TACK NON-TRACKING ⑤ TON 20430ED SAW CUT LF	05992	AGRICULTURAL LIMESTONE	TON	
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24970EC ASPHALT MATERIAL FOR TACK NON-TRACKING ⑤ TON 20430ED SAW CUT LF	06510	PAVE STRIPING - TEMP PAINT - 4 IN	LF	6
20430ED SAW CUT LF	08901	CRASH CUSHION TY VI CLASS BT TL2	EA	
	24970EC	ASPHALT MATERIAL FOR TACK NON-TRACKING (5)	TON	
23265ES717 PAVE MARK TY 1 TAPE STOP BAR-24 IN LF	20430ED	SAW CUT	LF	
	23265ES717	PAVE MARK TY 1 TAPE STOP BAR-24 IN	LF	

COUNTY OF	ITEM NO.	SHEET NO.
KNOTT	12-0243.0TH	R2A

NOTES:

ALL ASPHALT MIXTURES SHALL BE ESTIMATED AT 110 LBS. PER SO. YD. PER INCH OF DEPTH, UNLESS NOTED OTHERWISE.

- () ESTIMATED AT 115 LBS. PER SO. YD. PER INCH OF DEPTH
- ② FOR CONTROLLING DUST CAUSED BY MAINTAINING TRAFFIC ONLY.
- 3 APPROXIMATELY 0.43 ACRES
- (4) LANE & ROAD CLOSURES ARE INCIDENTAL TO MAINTAIN AND CONTROL TRAFFIC
- (5) ESTIMATED AT 0.70 LBS. PER SQ. YD.
- (6) 530 TONS FOR STREAM BANK STABILIZATION 100 TONS TO BE USED AS DIRECTED BY THE ENGINEER TO STABILIZE OTHER STREAM BANKS
- 7) TO BE USED AS DIRECTED BY THE ENGINEER TO MAINTAIN ACCESS
- (8) TO BE INSTALLED AT LOCATIONS IDENTIFIED IN STANDARD DRAWING NO. RBR-060
- 9 FOR CONSTRUCTION ENTRANCES
- (1) ESTIMATED AT 1.3 TONS PER CU. YD.
- ① TEMPORARY SIGNAL IS FULL COMPENSATION FOR EACH PHASE OF MOT

GENERAL SUMMARY

SCALE: N.T.S

USER: wmattingly DATE PLOTTED: October 19, 2022

GENERAL & SPECIAL NOTES

COUNTY OF	ITEM NO.	SHEET NO.
KNOTT	12-0243.0TH	R2B

PROPOSAL ATTACHMENTS

SPECIAL NOTE FOR FOR NON-TRACKING TACK COAT SPECIAL PROVISION FOR EMBANKMENT AT BRIDGE END BENT STRUCTURES SPECIAL NOTE FOR CONCRETE SEALING

160 N.G.S. (U.S.G.S.) BENCH MARKS

DO NOT DISTURB N.G.S. (U.S.G.S.) BENCH MARKS IN ANY MANNER UNLESS DIRECTED BY THE ENGINEER.

BEFORE YOU DIG

THE CONTRACTOR IS INSTRUCTED TO CALL 1-800-752-6007 TO REACH KY 811, THE ONE-CALL SYSTEM FOR INFORMATION ON THE LOCATION OF EXISTING UNDERGROUND UTILITIES. THE CALL IS TO BE PLACED A MINIMUM OF TWO (2) AND NO MORE THAN TEN (10) BUSINESS DAYS PRIOR TO EXCAVATION. THE CONTRACTOR SHOULD BE AWARE THAT OWNERS OF UNDERGROUND FACILITIES ARE NOT REQUIRED TO BE MEMBERS OF THE KY 811 ONE-CALL BEFORE-U-DIG (BUD) SERVICE. THE CONTRACTOR MUST COORDINATE
EXCAVATION WITH THE UTILITY OWNERS, INCLUDING THOSE WHOM DO
NOT SUBSCRIBE TO KY BII. IT MAY BE NECESSARY FOR THE CONTRACTOR
TO CONTACT THE COUNTY COURT CLERK TO DETERMINE WHAT UTILITY COMPANIES HAVE FACILITIES IN THE AREA.

429 WINTER CLOSEDOWN

ANY ASPHALT CONCRETE BASE AND/OR SURFACE COURSE USED AS A RIDING SURFACE EXPOSED TO TRAFFIC DURING WINTER CLOSEDOWN PERIODS SHALL CONTAIN NATURAL, CONGLOMERATE, CRUSHED SLAG, CRUSHED GRANITE OR CRUSHED SANDSTONE SAND IN THE PROPORTION OF NO LESS THAN 25% OF THE TOTAL COMBINED COARSE AND FINE AGGREGATE.

448 COMPACTION OF ASPHALT MIXTURES

WILL ACCEPT THE COMPACTION OF ASPHALT MIXTURES FURNISHED ON THIS PROJECT BY OPTION B ACCORDING TO SUBSECTIONS 402.03.02 AND 403.03.10 OF THE

EDGE KEY

THIS WORK INCLUDES CUITING OUT THE EXISTING ASPHALT SURFACE TO A MINIMUM DEPTH AND WIDTH AS DETAILED ELSEWHERE IN THE PLANS SO THAT THE NEW SURFACE MAY HEEL INTO THE EXISTING SURFACE. THE CONTRACT UNIT PRICE BID LINEAR FOOT FOR "EDGE KEY INCLUDES ALL NECESSARY MATERIALS, LABOR AND EQUIPMENT NECESSARY TO PERFORM THE WORK AND DISPOSE OF THE REMOVED ASPHALT MATERIAL.

650 STANDARD DRAWINGS

STANDARD DRAWINGS ARE NOT ATTACHED TO THESE PLANS. A STANDARD DRAWING BOOK AND THE HEADWALL SUPPLEMENTAL BOOK MAY BE OBTAINED FROM THE POLICY SUPPORT BRANCH OF THE DEPARTMENT OF ADMINISTRATIVE SERVICES IN FRANKFORT, KY. AT (502) 564-3670

CONSTRUCTION ENTRANCES

THE CONTRACTOR SHALL CONSTRUCT TEMPORARY CONSTRUCTION VEHICLE ACCESS ENTRANCES INTENDED TO REDUCE OFF- SITE TRACKING / WASHING OF SEDIMENT ONTO PUBLIC RIGHT OF WAY. THESE ENTRANCES SHALL BE CONSTRUCTED AT LOCATIONS APPROVED BY THE ENGINEER AND CONSISTING OF A MINIMUM OF 50 FEET IN LENGTH, 20 FEET IN WIDTH, AND 1 FOOT DEPTH OF CRUSHED AGGREGATE SIZE NO. 2 AND UNDERLAID WITH GEOTEXTILE FABRIC CLASS 2. QUANTITIES HAVE BEEN INCLUDED FOR TWO TEMPORARY CONSTRUCTION ENTRANCES.

TYPICAL SECTION

DIMENSIONS SHOWN ON THE TYPICAL SECTIONS FOR PAYEMENT WIDTH AND THICKNESS ARE NOMINAL OR TYPICAL DIMENSIONS. THE ACTUAL DIMENSIONS TO BE CONSTRUCTED MAY BE VARIED TO FIT EXISTING CONDITIONS AS DIRECTED OR APPROVED BY THE ENGINEER.

SPECIAL NOTE FOR EROSION PREVENTION AND SEDIMENT CONTROL

THE CONTRACTOR SHALL BE RESPONSIBLE FOR FILING THE KENTUCKY POLLUTION DISCHARGE ELIMINATION SYSTEM (KPDES) KYRIO PERMIT NOTICE OF INTENT (NOI) WITH THE KENTUCKY DIVISION OF WATER (DOW) AND ANY KPDES LOCAL MUNICIPAL SEPARATE STORM SEWER SYSTEM (MS4) PROGRAM THAT HAS JURISDICTION. THE NOI SHALL NAME THE CONTRACTOR AS THE FACILITY OPERATOR AND INCLUDE THE KYTC CONTRACT ID NUMBER (CID) FOR REFERENCE.

THE CONTRACTOR SHALL PERFORM ALL TEMPORARY EROSION/SEDIMENT CONTROL FUNCTIONS INCLUDING: PROVIDING A BEST MANAGEMENT PRACTICE (BMP) PLAN, CONDUCTING REQUIRED INSPECTIONS, MODIFYING THE BMP PLAN DOCUMENTS AS CONSTRUCTION PROGRESSES AND DOCUMENTING THE INSTALLATION AND MAINTENANCE OF BMPS IN CONFORMANCE WITH THE KPDES KYRIO PERMIT DATED SEPTEMBER 30, 2003 OR A PERMIT RE-ISSUED TO REPLACE THE KYRIO PERMIT. THIS WORK SHALL BE CONDUCTED IN CONFORMANCE WITH THE REQUIREMENTS OF SECTION 213 OF KYTC 2008 DEPARTMENT OF HIGHWAYS, STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION.

CONTRARY TO SECTION 213.03.03, PARAGRAPH 2, THE ENGINEER SHALL CONDUCT INSPECTIONS AS NEEDED TO VERIFY COMPLIANCE WITH SECTION 213 OF KYTC 2012 DEPARTMENT OF HIGHWAYS, STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION. THE ENGINEER'S INSPECTIONS SHALL BE PERFORMED A MINIMUM OF ONCE PER MONTH AND WITHIN SEVEN DAYS AFTER A STORM OF 1/2 INCH OR GREATER. COPIES OF THE ENGINEER'S INSPECTIONS SHALL NOT BE PROVIDED TO THE CONTRACTOR UNLESS IMPROVEMENTS TO THE BMP'S ARE REQUIRED. THE CONTRACTOR SHALL INITIATE CORRECTIVE ACTION WITHIN 24 HOURS OF ANY REPORTED DEFICIENCY AND COMPLETE THE WORK WITHIN 5 DAYS. THE ENGINEER SHALL USE FORM TC 63-61 A FOR THIS REPORT. INSPECTIONS PERFORMED BY THE ENGINEER DO NOT RELIEVE THE CONTRACTOR OF ANY RESPONSIBILITY FOR COMPLIANCE WITH THE KPDES PERMIT.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR APPLYING "GOOD ENGINEERING PRACTICES"AS REQUIRED BY THE KPDES PERMIT. THE CONTRACTOR MAY USE ANY TEMPORARY BMPS WITH THE APPROVAL OF THE KYTC ENGINEER.

THE CONTRACTOR SHALL PROVIDE THE ENGINEER COPIES OF ALL DOCUMENTS REQUIRED BY THE KPDES PERMIT AT THE TIME THEY ARE PREPARED.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE EXAMINATION OF THE SOILS TO BE ENCOUNTERED AND MAKE HIS OWN INDEPENDENT DETERMINATION OF THE TEMPORARY BMPS THAT WILL BE REQUIRED TO ACCOMPLISH EFFECTIVE EROSION PREVENTION AND SEDIMENT CONTROL.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR FILING THE KPDES PERMIT NOTICE OF TERMINATION (NOT) WITH THE KENTUCKY DOW AND ANY LOCAL MS4 PROGRAM THAT HAS JURISDICTION. THE NOT SHALL BE FILED AFTER THE ENGINEER AGREES THAT THE PROJECT IS STABILIZED OR THE PROJECT HAS BEEN FORMALLY ACCEPTED.

SPECIAL NOTE FOR OVERHEAD UTILITIES

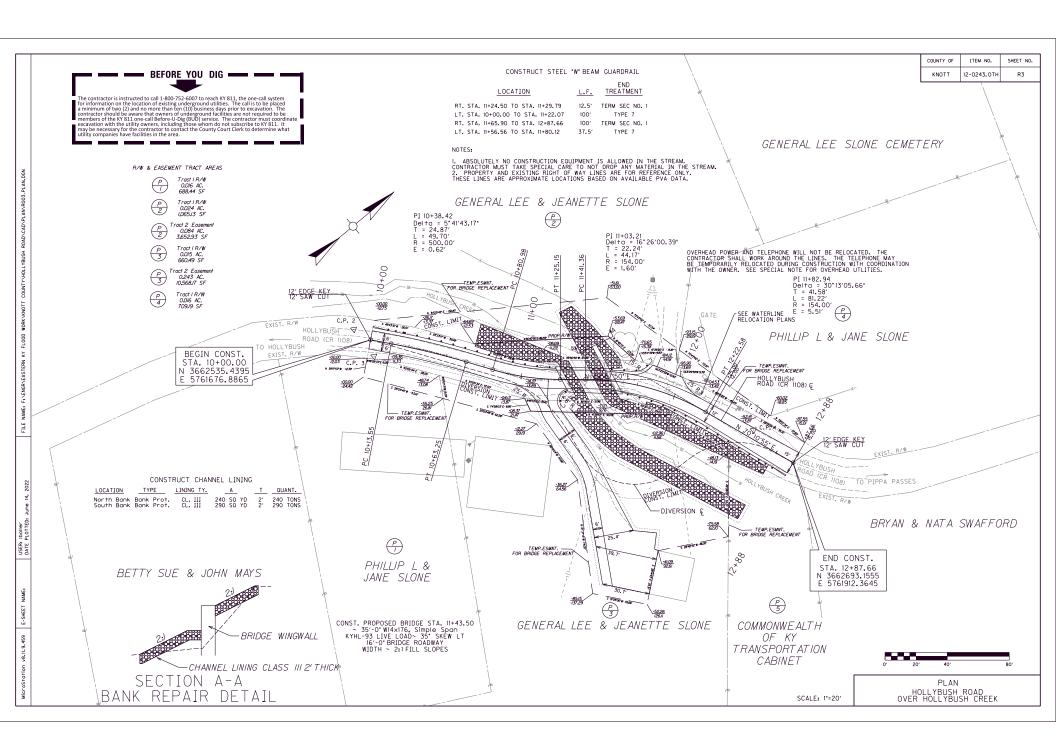
THE CONTRACTOR MUST WORK SAFELY AROUND THE POWERLINES TO COMPLETE THE CONSTRUCTION OF THIS PROJECT. THE CONTRACTOR SHALL COORDINATE WITH THE KY POWER COMPANY AND TV SERVICES PRIOR TO BEGINNING CONSTRUCTION ACTIVITIES, CONTACT TV SERVICES IF THEIR LINE NEEDS TO BE RELOCATED. RELOCATION OF THE POWER LINE IS NOT AN OPTION. THE CONTRACTOR SHALL CONTACT KY POWER 10 DAYS PRIOR TO PERFORMING CONSTRUCTION ACTIVITIES THAT REQUIRE KY POWER TO DEENERGIZE ELECTRICAL LINES. THE CONTRACTOR SHALL ORGANIZE CONSTRUCTION ACTIVITIES TO MINIMIZE THE TIME REQUIRED FOR DEENERGIZED LINES. THE CONTRACTOR MUST TAKE CARE TO NOT DAMAGE ANY OVERHEAD UTILITIY LINES. THE KY POWER COMPANY WILL PLACE A SHIELD AROUND THE POWER LINE UPON CONSTRACTORS REQUEST.

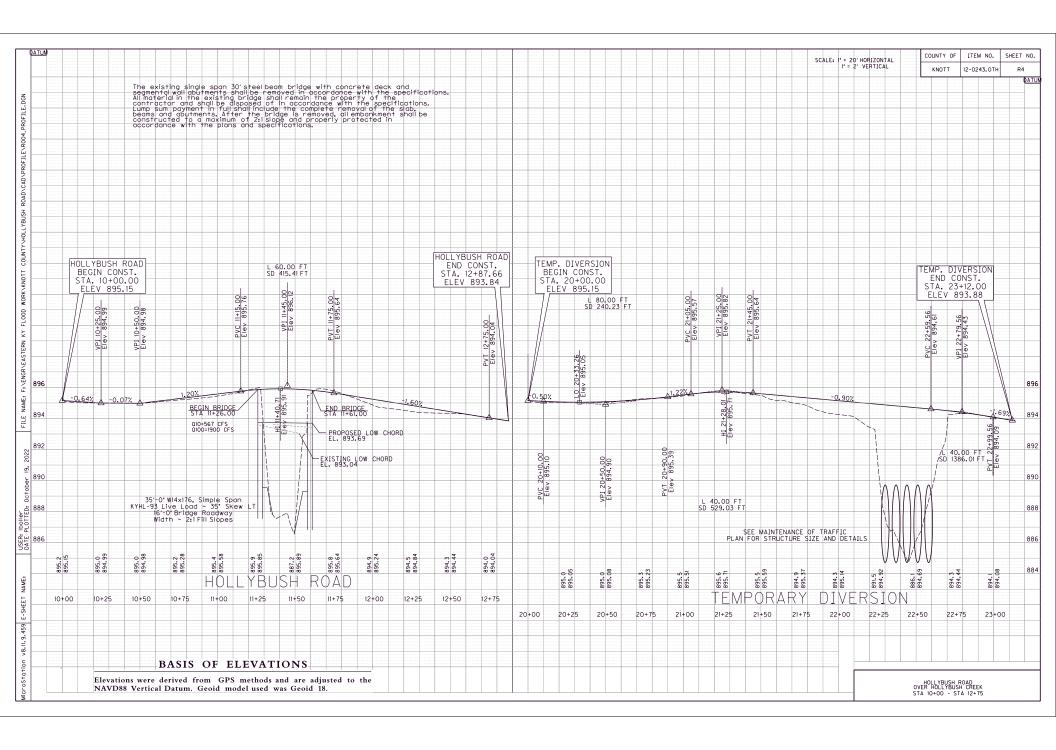
THE CONTRACTOR SHALL CONTACT KY POWER 10 DAYS PRIOR TO PERFORMING CONSTRUCTION ACTIVITIES THAT REQUIRE KY POWER TO DEENERGIZE ELECTRICAL LINES. THE CONTRACTOR SHALL ORGANIZE CONSTRUCTION ACTIVITIES TO MINIMIZE THE TIME REQUIRED FOR DEENERGIZED LINES. THE CONTRACTOR MUST TAKE CARE TO NOT DAMAGE ANY OVERHEAD UTILITIY LINES.

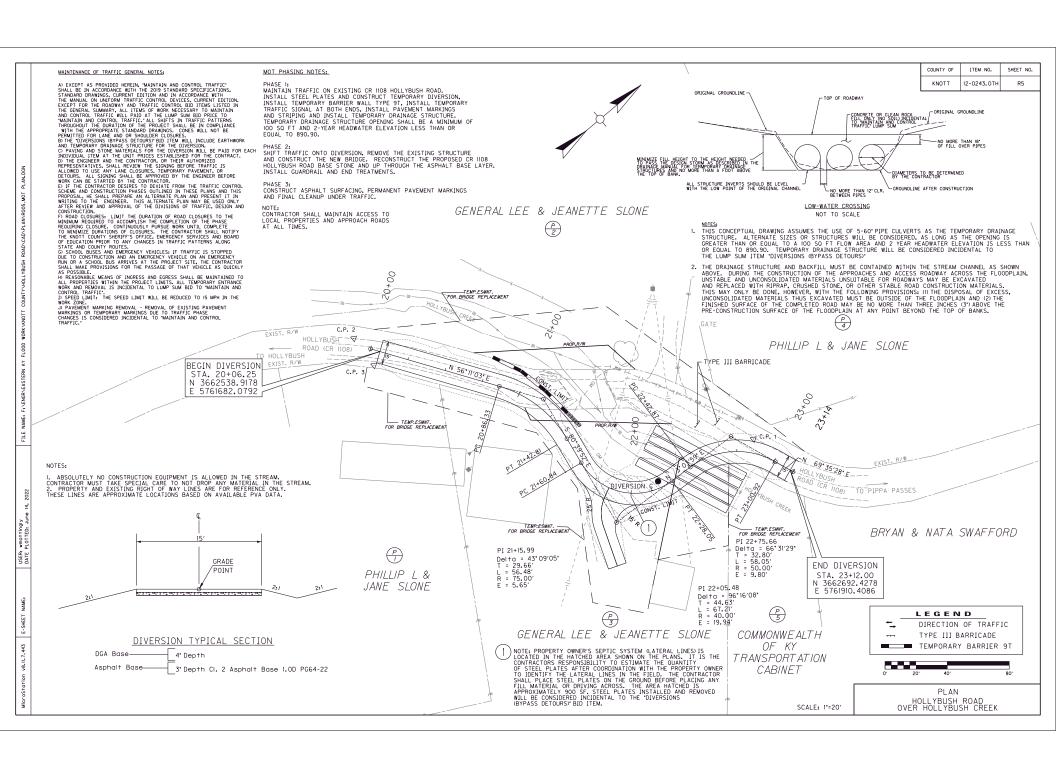
WORKING WITH OR NEAR POWER LINES CAN EXPOSE WORKERS TO ELECTRICAL HAZARDS, BUT THESE DANGERS CAN BE AVOIDED THROUGH SAFE WORK PRACTICES. THESE PRACTICAL STEPS CAN PREVENT INJURIES FROM CONTACT WITH POWER LINES. - CONDUCT A HAZARD ASSESSMENT TO IDENTIFY AND ADDRESS POTENTIAL SAFETY HAZARDS BEFORE WORK BEGINS.

- ASK THE ELECTRIC COMPANY TO DE-ENERGIZE AND GROUND OVERHEAD POWER LINES. EDUCATE WORKERS ON SAFETY PROCEDURES AND REQUIREMENTS.
- KNOW THE SAFE WORKING DISTANCE FOR WORKERS AND EQUIPMENT. - USE NON-CONDUCTIVE WOOD OR FIBERGLASS LADDERS.
- WEAR PERSONAL PROTECTIVE EQUIPMENT, SUCH AS RUBBER INSULATING GLOVES AND

INSULATING SLEEVES, AND INDUSTRIAL PROTECTIVE HELMETS. FOR MORE INFORMATION ON RECOGNIZING HAZARDS FROM ENERGIZED POWER LINES, VISIT OSHA'S ELECTRICAL SAFETY AND HEALTH TOPICS PAGE.







PAVEMENT CROSS-SECTION

TRAVELED	TYFE OF	NO	N-STATE PR	IMARY RO	STATE PRIMARY ROUTES			
WAY	PAVEMENT STRIPING	< 1000 ADT		>= 1000 ADT		ANY ADT		
2		WIDTH	MATERIAL	WIDTH	MATERIAL	WIDTH	MATERIAL*	
< 16' ④	EDGELINE STRIPES ONLY	4"	PAINT	4"	PAINT	6"	THERMO (ASHPALT) TYPE I TAPE (CONCRETE)	
16' TO < 20'	EDGELINE STRIPES ONLY OR CENTERLINE STRIPE ONLY	4"	PAINT	4"	PAINT	6"	THERMO (ASHPALT) TYPE I TAPE (CONCRETE)	
>=20' ③	CENTERLINE AND EDGELINE STRIPES	4" (S	PAINT	6"	PAINT	6"	THERMO (ASHPALT) TYPE I TAPE (CONCRETE)	

*OTHER DURABLE NON-WATERBORNE MARKINGS MAY BE USED WITH APPROVAL FROM THE DIVISION OF TRAFFIC OPERATIONS.

~ NOTES ~

- 1. INSTALL PAVEMENT STRIPING ON TWO LANE, TWO WAY ROADWAYS AS DETAILED IN THE ABOVE TABLE AND IN ACCORDANCE WITH THE PAVEMENT MARKINGS AND DELINEATION CHAPTER OF THE TRAFFIC OPERATIONS GUIDANCE MANUAL. CONTACT THE DIVISION OF TRAFFIC OPERATIONS FOR ADDITIONAL GUIDANCE IF NECESSARY.
- ② THE TRAVELED WAY IS THE PORTION OF ROADWAY FOR THE MOVEMENT OF VEHICLES, EXCLUSIVE OF THE SHOULDERS.
- 3 ON TWO LANE. TWO WAY ROADWAYS THAT HAVE A TOTAL PAVEMENT WIDTH (W) THAT IS 20 FT OR GREATER, BUT LESS "HAN 22 FT, EDGELINE RUMBLE STRIPS ARE NOT A STANDARD APPLICATION, BUT THEY MAY BE INSTALLED. THE DIVISION OF TRAFFIC OPERATIONS IS AVAILABLE TO ASSIST WITH THE DETERMINATION OF WHETHER OR NOT TO INSTALL ESGELINE RUMBLE STRIPS ON PAVEMENT WIDTHS LESS THAN 22 FT, AS WELL AS THE DIMENSION AND PLACEMENT DETAILS OF THE RUMBLE STRIPS AND PAVEMENT STRIPING.

ON TWO LANE, TWO WAY ROADWAYS THAT HAVE A TOTAL PAVEMENT WIDTH (W) THAT IS 22 FT OR GREATER, BUT LESS "HAN 34 FT, INSTALL PAVEMENT STRIPING AS DETAILED IN THE ABOVE TABLE AND IN CONJUNCTION WITH CENTERLINE AND EDGELINE RUMBLE STRIPS AS DETAILED ON TPA-120.

ON TWO LANE, TWO WAY ROADWAYS THAT HAVE A TOTAL PAVEMENT WIDTH (W) THAT IS 34 FT OR GREATER, INSTALL PAVEMENT STRIPING AS DETAILED IN THE ABOVE TABLE AND IN CONJUCTION WITH CENTERLINE AND SHOULDER RUMBLE STRIPS AS DETAILED ON TPR-125.

- GOGELINES MAY BE OMITTED FROM ROADWAYS WITH A TRAVELED WAY WIDTH LESS THAN 16 FEET WITH THE APPROVAL OF THE DIVISION OF TRAFFIC OPERATIONS.

 EDGELINES MAY BE OMITTED ON NON-STATE PRIMARY ROUTES WITH A TRAVELED WAY WIDTH GREATER THAN OR EQUAL TO 20 FEET AND
- (3) EDGELINES MAY BE 3MITTED ON NON-STATE PRIMARY ROUTES WITH A TRAVELED WAY WIDTH GREATER THAN OR EQUAL TO 20 FEET AND AN ADT LESS THAN 1,000.
- 6. EDGELINES MAY BE OMITTED, BASED ON ENGINEERING JUDGMENT, IN AREAS WHERE THE PAVEMENT EDGE IS DELINEATED BY PHYSICAL OBJECTS SUCH AS CURBS, PARKING SPACES, OR OTHER MARKINGS. EDGELINES SHOULD BE INSTALLED ON ROADWAYS WITH CURB AND GUTTER IF THE POSTED SPEED LIMIT IS 45 MMP OR GREATER.

DRAWING NOT TO SCALE

USE WITH CUR. STD. DWGS.

TPR-120 & TPR-125

SUBMITTED Dim A 06-09-21

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

DRAWING TITLE: SEPIA 017 - PAVEMENT STRIPING DETAILS FOR TWO LANE TWO WAY ROADWAYS

OpenRoads Designer v10.16.0.80

USER: cwillmerdinge

DATE PLOTTED: 8/7/2015 9:53:17 AM

FILE NAME: C:\PWWORK\KYTC_CWILLMELDINGER\D1859968\SEPIA017

HOLLYBUSH ROAD OVER HOLLYBUSH CREEK SEPIA 017

TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS

KNOTT COUNTY CR-1108 (HOLLYBUSH ROAD) **OVER HOLLYBUSH CREEK** STA. 11+43.50

	ESTIMATE OF QUANTITIES													
	BID ITEM CODE	08100	08104	08150	08151	08003	08033	08046	08039	08160	02231	03299	23378EC	23539EC
	BID ITEM	CONCRETE-CLASS A	CONCRETE-CLASS AA	STEEL	STEEL REINFORCEMENT- EPOXY COATED	FOUNDATION	TEST PILES	PILES-STEEL HP12X53	PRE-DRILLING FOR PILES	STRUCTURAL STEE	STRUCTURE GRANULAR BACKFILL	ARMORED EDGE FOR CONCRETE	CONCRETE SEALING	BRIDGE RAIL
	UNIT	CUYD	CUYD	LB	LB	LS	LF	LF	LF	LS	CUYD	LF	SQFT	LF
	END BENT#1	29.4		2432			19	57	20		76.4	19	407	
l w	END BENT #2	47.3		3740			19	95	30		110.0	19	526	
ΙĒ														
13														
I E														
SUBSTRUCTURE														
I۳														
	SUPERSTRUCTURE		30.7		5212					1			1089	
В	RIDGE TOTALS	76.7	30.7	6172	5212	1	38	152	50	1	186.4	28	2022	70

NOTE: The total estimated weight for structural steel is 14 171 LB.

S3 Layout S4 Subsurface Data S5 Foundation Layout S11 Composite Steel Beam Superstructures General Notes S12 Composite Steel Beam Superstructures Beam Tables S13 Composite Steel Beam Superstructures Slab Details S14 Comp. St. Bm. Super. Framing Plan / Diaphragms S15 Construction Elevations SPECIAL NOTES Special Note for Concrete Sealing Special Note for Hot-Dip Galvanizing Steel SPECIAL PROVISIONS 69 Embankment at Bridge End Bent Structures, c.e. STANDARD DRAWINGS BGX-006-10 Stencils for Structures BGX-012-02 Geotechnical Legend BGX-022 Joint Waterproofing Railing System Type T631 Details BJE-001-14 Armored Edges BPS-003-09 HP12x53 Steel Pile Composite Steel Beam Superstructures General Notes Composite Steel Beam Superstructures Beam Tables Composite Steel Beam Superstructures Slab Details Composite Steel Beam Superstructures Framing Plan / Diaphragms **SPECIFICATIONS** 2019 Standard Specifications for Road and Bridge Construction ASHTO LRFD Bridge Design Specifications, 9th Edition, 2020 2-0243.OTI KNOTT CR-1108 28598

INDEX OF SHEETS Description

S1 Title Sheet & Quantities

S2 General Notes

FEMA Bridge: D023A 4663-DR

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

HMB PROFESSIONAL ENGINEERS, INC.

DATE: 10/15/22 CHECKED BY DESIGNED BY: L. BOLLER . REID DETAILED BY: L. BOLLEF B, REID

TITLE AND ESTIMATE OF QUANTITIES HOLLYBUSH CREEK

GENERAL NOTES

SPECIFICATIONS: All references to the Specifications are to the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction with current Supplemental Specifications. All references to the AASHTO Specifications are to the current edition of the AASHTO LRFD Bridge Design Specs, with interims.

DESIGN LOAD: This bridge is designed for a KYHL-93 live load. The KYHL-93 live load is arrived at by increasing the standard HL-93 truck and lane loads as specified in the AASHTO Specifications by 25%.

FUTURE WEARING SURFACE: This Structure is designed for a 15 PSF future wearing

DESIGN STRESSES:

Concrete Class "A" ~ f'c = 3500 psi Concrete Class "AA" ~ f'c = 4000 psi Steel Reinforcement ~ Fy = 60,000 psi

DESIGN METHOD: All reinforced concrete members are designed by the load and resistance factor method as specified in the current AASHTO Specifications.

MATERIAL SPECIFICATIONS: AASHTO Specifications or ASTM, current edition, as designated below shall govern the materials furnished.

STEEL REINFORCEMENT, GRADE 60

AASHTO M-203, GRADE 270

STRUCTURAL STEEL, 36,000 PSI MIN. YIELD

AASHTO M-270, GRADE 36

PREFORMED EXPANSION JOINT MATERIAL: Preformed Cork Expansion Joint Material shall conform to subsection 807.04.02 (Type II) of the Kentucky Department of Highways Standard Specifications.

CONCRETE: Class "AA" Concrete is to be used throughout the superstructure and in the portions of the substructure above the tops of caps. Class "A" Concrete is to be used in the substructure below the caps. Prestressed beam concrete shall be in accordance with the plans and specifications.

REINFORCEMENT: Dimensions shown from the face of concrete to bars are to center of bars unless otherwise shown. Spacing of bars is from center to center of bars. Clear distance to face of concrete is 2', unless otherwise noted. Any reinforcing bars designated by suffix (e) in the plans shall be epoxy coated in accordance with section 811.10 of the Standard Specifications. Any reinforcing bars designated by suffix (s) in a bill of reinforcement shall be considered a stirrup for purposes of bend diameters.

CONSTRUCTION IDENTIFICATION: The names of the Prime Contractor and the Sub-Contractor shall be imprinted in the concrete with 1-inch letters at a location designated by the engineer. The Contractor shall furnish all plans, equipment, and labor necessary to do the work for which no direct payment will be made.

BEVELED EDGES: Bevel all exposed edges 3/4, unless otherwise noted.

SITE INSPECTION: The contractor shall familiarize themselves with all conditions at the bridge site. Submission of a bid shall be considered evidence that the contractor has performed a site visit and is familiar with all aspects of the existing bridge and requirements in these plans. The cabinet will not consider any claims due to changed site conditions.

DAMAGE TO STRUCTURE: The contractor shall bear all responsibility and expense for any and all damage to the structure during repair work, even to the removal and replacement of the entire structure, should it be damaged by the contractors COMPLETION OF THE STRUCTURE: The Contractor is required to complete the structure in accordance with the plans and specifications. Material labor or construction operations, not otherwise specified, are to be included in the bid Item most appropriate to the work involved. This may include cofferdams, shoring, excavations, backfilling, removal of all or parts of existing structures, phase construction, incidental materials, labor or anything else required to complete the structure.

SHOP DRAWINGS: Fabricators shall submit all required shop plans by e-mail to the design engineer for review (xxx@docs.e-builder.net). These submissions shall depict the shop plans in PDF format as either II'x I7' or 22' x 36' sheets. Designers will make review comments on these electronic submissions as needed and return them to the fabricator. Upon reconciliation of the designer's comments, files shall be returned to the designer. Each sheet will be electronically stamped by the designer and plans will be forwarded to the Construction Management Team for distribution. Only plans submitted directly to the Construction Management Team will be distributed, and only plans electronically stamped 'Distributed by Construction Management Team are to be used for fabrication. While this process does not require the submission of paper copies. Construction Management Team reserves the right to require such copies on a case by case basis.

When any changes in the design plans are proposed by the fabricator or supplier, the shop drawings reflecting these changes shall be submitted to the consultant through the contractor.

HITH LITES. The contractor shall be responsible for locating any and all existing utilities prior to excavation of material or installation of guardrail or other construction activities that may involve utilities (overhead or underground).

VERIFYING FIELD CONDITIONS: The contractor shall field verify all dimensions before ordering material. New material that is unsuitable because of variations in the existing structure shall be replaced at the contractor's expense.

DIMENSIONS: Dimensions are for a normal temperature of 60 degrees Fahrenheit. Layout dimensions are horizontal dimensions.

SUPERSTRUCTURE SLAB: The superstructure slab shall be poured continuously from end to end of slab before the concrete is allowed to set.

MASTIC TAPE: Mostic Tope used to seal joints is to meet the requirements of ASTM C-877 Type I, II, and III. The joint is to be covered with 12° wide mastic tape. Prior to application, the joint surface shall be clean and free of dirt, debris, or deleterious material. Primer, if required by the tape manufacturer, shall be applied for a minimum width of 9 on each side of the joint.

Mastic Tape shall be either: MGSTIC Tape Statute that: EZ-Wrap Rubber by Press-seal Casket Corporation, SealWrap by Mar Mac Manufacturing Co. Inc., Cadilloc by The UP Rubber Co. Inc., or approved equal.

Mastic tape shall cover the joint continuously unless otherwise shown in the plans. Mastic Tape shall be spliced by taping a minimum of 6' and in accordance with the manufacturer's recommendations with the overlap running

The cost of labor, materials, and incidental items for furnishing and installing Mastic Tape shall be considered incidental to the unit price bid for Concrete Class "AA" and no separate measurement of payment shall be made.

TEMPORARY SUPPORTS: Temporary supports or shoring will not be permitted under the beams when pouring the concrete deck slab or when taking 'top of beam'

ARMORED EDGE: Fabricate armored edge to match cross slope and parabolic crown at each end of bridge.

FOUNDATION PREPARATION: Foundation Preparation shall be in accordance with Section 603 of the Specifications.

CONCRETE SEALER: Apply concrete sealer in accordance with the Special Note for Concrete Sealing to the superstructure limits shown on the superstructure detail sheet and all exposed surfaces of the end bents to 6" below finished ground line.

PILING: Piling shall be driven to practical refusal as defined on the pile record sheet. Test piles shall be driven where designated on the plans to determine the length of pile required. All test piles shall be accurately located so that they may be used in the finished structure. Contrary to the standard drawings for steel piling, mill test reports are not required to

JOINT WATERPROOFING: In addition to the standard drawing, all joints are to be sealed including those between beams.

FEMA Bridge: D23A 4663-DR

COMMONWEALTH OF KENTUCKY () DEPARTMENT OF HIGHWAYS

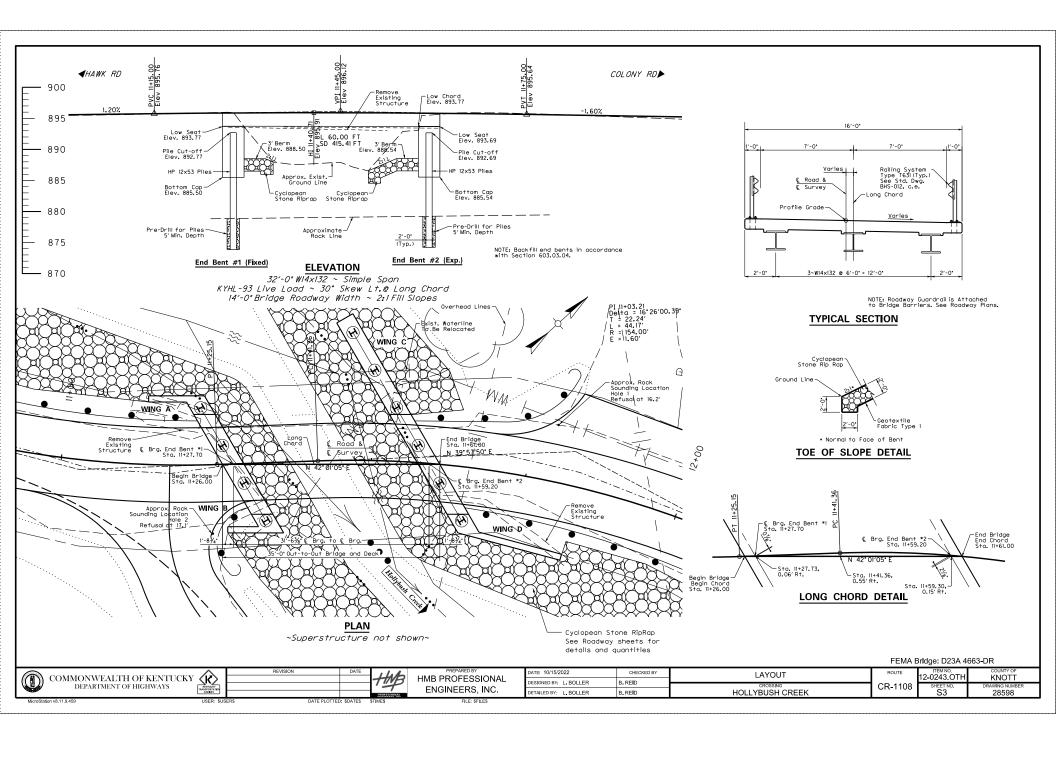
TRANSPORTATED CASINET

HMB PROFESSIONAL ENGINEERS, INC.

DATE: 10/15/2022 CHECKED BY DESIGNED BY: L. BOLLER . REID DETAILED BY: L. BOLLER B, REID

GENERAL NOTES HOLLYBUSH CREEK

2-0243 OT KNOTT CR-1108 S2 28598



Drilling Firm: Kentucky Transportation Cabinet For: Division of Structural Design Geotechnical Branch

Description

Project ID: 060C00022N

Item Number: Hole Number 1

Surface Elevation ______

Total Depth 16.2'

Location + 'Lt

Lithology

Elevation Depth

10.0

14.6

16.2

DRILLER'S SUBSURFACE LOG

Start Date <u>08/20/2022</u>

End Date <u>08/20/2022</u>

Latitude(83) 37.346094 Longitude(83) <u>-82.857996</u>

Depth (ft)

Std/Ky RQD

Rec.

Rec (ft)

Knott - HOLLYBUSH RD

HOLLYBUSH CREEK

Overburden

Rock Core

(Refusal)

mmediate Water Depth ____NA_

Static Water Depth NA

Driller <u>James Roark</u>

Brown, moist, silty clay with sandstone boulders.

Gray, wet, sandy clay with rock fragments.

Gray, sandstone.

(Bottom of Hole 16.2') (Refusal @ 16.2)

Printed: 8/22/22 Page 1 of 1

Remarks

Project Type: Structure Bridge

Hole Type sounding

Sample Type

SDI (JS)

Rig_Number __

Project Manager: _

SPT

Rec (%)

Drilling Firm: Kentucky Transportation Cabinet For: Division of Structural Design Geotechnical Branch

DRILLER'S SUBSURFACE LOG

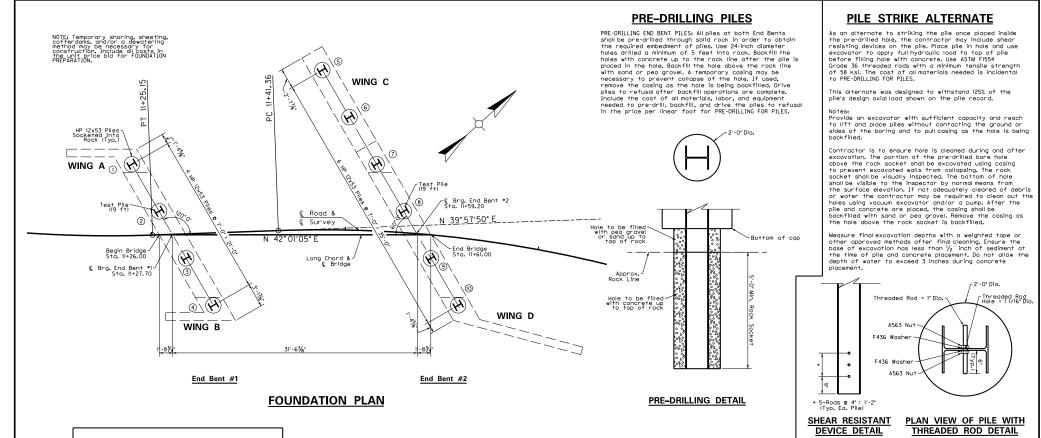
Printed: 8/22/22

Page 1 of 1

Project II Item Nur		00022N	Knott - HOLLYBUSH RD HOLLYBUSH CREEK					Project Type: Structure Bridge Project Manager: _				
Hole Numb Surface Ele Total Depth Location	evation <u>'</u> h <u>17.1'</u>		Immediate Water Depth	NA_	End D	oate <u>08/20/20</u> ate <u>08/20/20</u> de(83) <u>37.34</u> ude(83) <u>-82</u>	022	Hole Type <u>sounding</u> Rg_Number <u></u>				
Litholo		_		Overburden	Sample No.	Depth (ft)	Rec.	SF Blo	PT ws	Sample Type	2	
Elevation	Depth	Description	n	Rock Core	Std/Ky RQD	Run (ft)	Rec (ft)	Re (%		SDI (JS)	Remarks	
5		Mediu	n stiff, brown, clay with bou	ulders.							į.	
10 - -	10.0	Gray	wet, sand with rock fragme	ents.							1	
15	17.1		Gray, sandstone.	(Refusal)							1	
- 20 - -			(Bottom of Hole 17.1') (Refusal @ 17.1)								2	
<u>25</u>											2	
<u>30</u> -											<u>31</u>	
35											3:	
40 - -											41	
45 -											4:	
- 50											50	

50			50 50					50	SUBSI
							FEMA E	Brldge: D23A 46	63-DR
COMMONWEALTH OF KENTLICKY A	REVISION	HAB	PREPARED BY HMB PROFESSIONAL	DATE: 10/15/2022	CHECKED BY	SUBSURFACE DATA	ROUTE	12-0243.OTH	COUNTY OF KNOTT
COMMONWEALTH OF KENTUCKY OF HIGHWAYS PARTY PROPERTY OF HIGHWAYS		TIME		DESIGNED BY: L. BOLLER	B. REID	CROSSING	CR-1108	SHEET NO.	DRAWING NUMBER
THAMSTORIATED		PROFESSIONAL	ENGINEERS, INC.	DETAILED BY: L. BOLLER	B. REID	HOLLYBUSH CREEK	011-1100	S4	28598

SUBSURFACE DATA



PILE RECORD FOR POINT BEARING PILES

PILE NO.	PILE CUT-OFF ELEVATION (FEET)	TIP OF PILE ELEVATION AS DRIVEN (FEET)	LENGTH OF PILE IN PLACE (FEET)	DESIGN AXIAL LOAD (TONS)
1	892.77			73
2	892.77			73
3	892.77			73
4	892.77			73
5	892.69			73
6	892.69			73
7	892.69			73
8	892.69			73
9	892.69			73
10	892.69			73

FIELD DATA

FOR EACH PILE, THE PROJECT ENGINEER SHALL RECORD THE FOLLOWING ON THIS SHEET: PILE LENGTH IN PLACE AND POINT OF PILE ELEVATION AS DRIVEN. SUBMIT THIS RECORD TO:

DIRECTOR, DIVISION OF STRUCTURAL DESIGN ROOM *322 200 MERO STREET FRANKFORT, KY. 40622-0001

THIS PILE RECORD DOES NOT REPLACE OTHER PILE RECORDS THE PROJECT ENGINEER IS REQUIRED TO KEEP AND SUBMIT.

USE HP 12X53 IN ACCORDANCE WITH BPS-003, C.E. USE GRADE 50 STEEL H-PILES WITH PILE POINTS FOR END BEARING PILES.

DEFINITIONS OF TERMS

PILE CUT-OFF ELEVATION: Elevation of the top of pile in the finished structure. PILE LENGTH IN PLACE: Actual pile length below the Pile Cut-Off Elevation in the finished structure.

POINT OF PILE ELEVATION AS DRIVEN: Actual point of pile elevation in the finished structure.

<code>DESIGN AXIAL LOAD:</code> Load carried by each pile as estimated from structural design calculations for Factored LRFD Loadings.

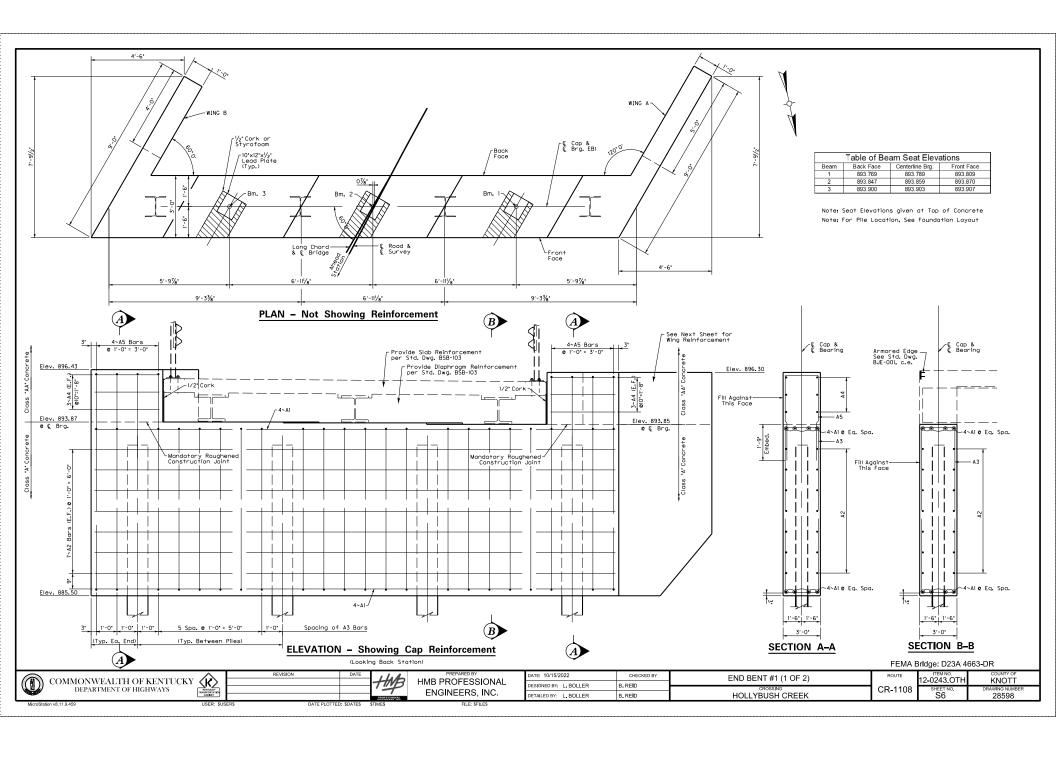
CALCULATED FIELD BEARING: Contrary to Section 604.03.07 of the Standard Specifications, in place bearing values are not required for piles bearing on rock when driven to practical refusal.

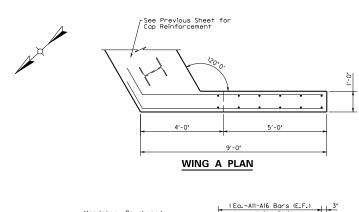
DRIVING CRITERIA: Drive point bearing piles to practical refusal.

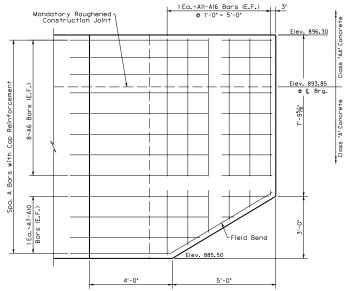
PRACTICAL REFUSAL; FOR THIS PROJECT MINIMUM BLOW REQUIREMENTS ARE REACHED AFTER TOTAL PENETRATION BECOMES ½* OR LESS FOR 10 CONSECUTIVE BLOWS. PRACTICAL REFUSAL IS OBTAINED AFTER THE PILE IS STRUCK AN ADDITIONAL 10 BLOWS WITH TOTAL PENETRATION OF ½* OR LESS. IMMEDIATELY CEASE DRIVING OPERATIONS IF THE PILE VISIBLY YIELDS OR BECOMES DAMAGED DURING DRIVING. DRIVE ADDITIONAL PRODUCTION AND TEST PILES IF DIRECTED BY THE ENGINEER.

FEMA Bridge: D23A 4663-DR

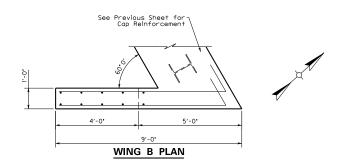


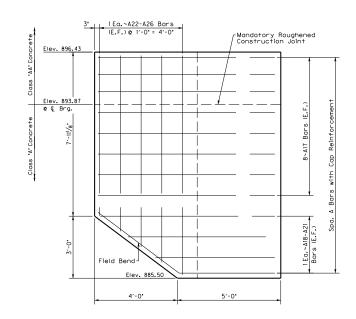












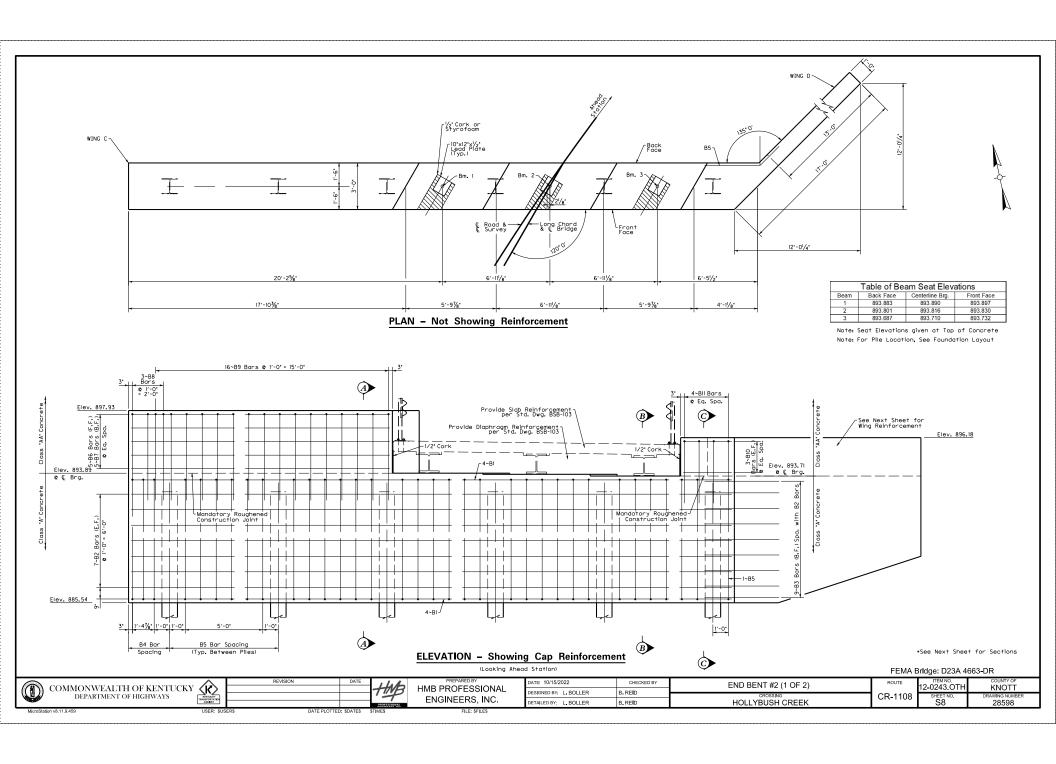
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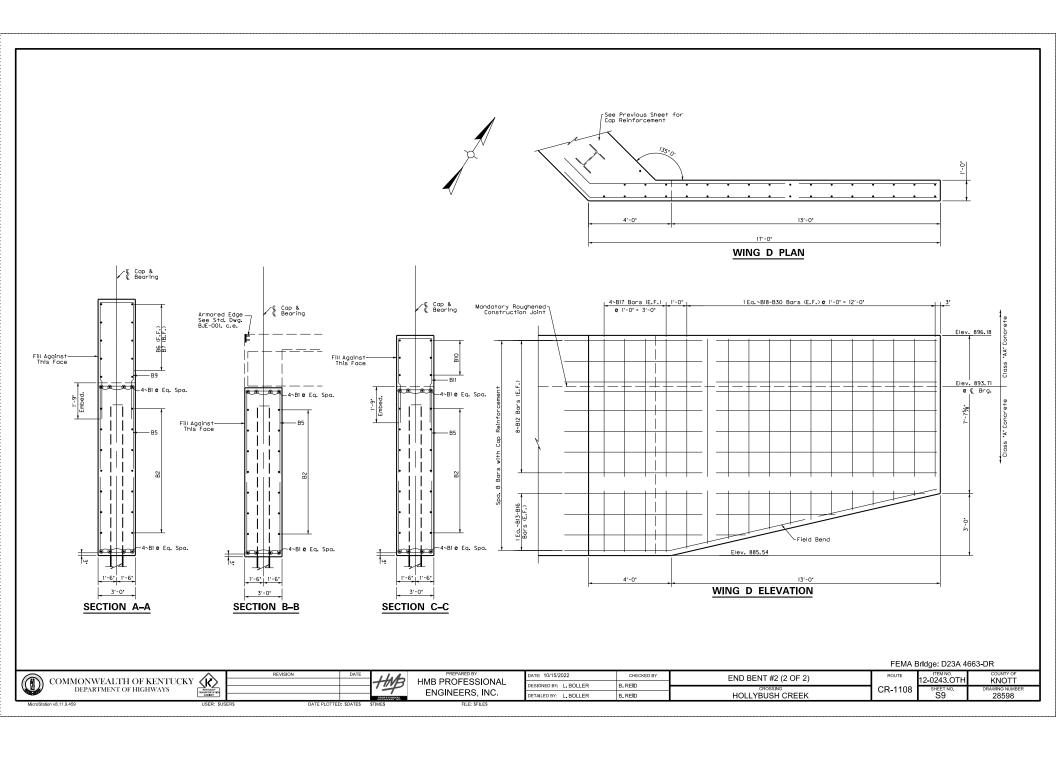
FEMA Brldge: D23A 4663-DR

COMMONWEALTH OF KENTUCKY
DEPARTMENT OF HIGHWAYS

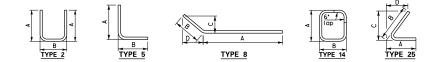
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END BENT BILL OF REINFORCEMENT														
MARK	TYPE	NO.	SIZE	FT	IN	LOCATION	FT	A IN	FT	B IN	FT	C IN	FT	D IN
A1	STR	8	#6	25	2	EB1 CAP HORIZONTAL	+ · ·		···		 			<u> </u>
A2	STR	14	#5	25	2	EB1 CAP FACES	_							-
A3	14	22	#6	22	10	EB1 CAP STIRRUP	7	10	3	1				-
A4	STR	12	#5	3	1	EB1 CAP WING HORIZ	+		_					-
A5	2	8	#6	11	7	EB1 CAP WING STIRRUP	4	2	3	1			_	-
A6	8	16	#6	10	2	WING A HORIZONTAL	8	8	1	6	1	4	0	9
A7	8	2	#5	10	0	WING A HORIZONTAL	8	6	1	6	1	4	0	9
A8	8	2	#5	8	6	WING A HORIZONTAL	7	0	1	6	1	4	0	1
A9	8	2	#5	6	9	WING A HORIZONTAL	5	3	1	6	1	4	0	1
A10	8	2	#5	11	0	WING A HORIZONTAL	9	6	1	6	1	4	Ö	1
A11	STR	2	#5	10	5	WING A VERTICAL	Ť	Ť		Ť			Ť	T.
A12	STR	2	#5	9	11	WING A VERTICAL	+						-	\vdash
A13	STR	2	#5	9	4	WING A VERTICAL							_	\vdash
A14	STR	2	#5	8	9	WING A VERTICAL	+						_	\vdash
A15	STR	2	#5	8	2	WING A VERTICAL	+						_	\vdash
A16	STR	2	#5	7	7	WING A VERTICAL WING A VERTICAL	+						_	\vdash
A17	25	16	#6	9	6	WING B HORIZONTAL	8	0	1	6	1	3	0	١,
A18	25	2	#5	9	5	WING B HORIZONTAL	7	11	1	6	1	3	0	
A19	25	2	#5	8	1	WING B HORIZONTAL	6	7	1	6	1	3	0	
A19	25	2	#5	6	9	WING B HORIZONTAL	5	3	1	6	1	3	0	
A21	25	2	#5	10	6	WING B HORIZONTAL	9	0	1	6	1	3	0	
A22	STR	2	#5	10	7	WING B HORIZONTAL WING B VERTICAL	9	U	-	- 6	-	3	-	Η.
													_	\vdash
A23	STR	2	#5	10 9	0	WING B VERTICAL	_							-
A24	STR		#5			WING B VERTICAL	-						_	⊢
A25	STR	2	#5	8	6	WING B VERTICAL	-						_	⊢
A26	STR	2	#5	7	9	WING B VERTICAL	+				_		_	\vdash
B1	STR	8	#6	38	8	EB2 CAP HORIZONTAL	-				_		_	⊢
B2	STR	14	#5	38	8	EB2 CAP FACES	-							
B3	8	9	#5	5	5	EB2 CAP BEND HORIZ.	3	8	1	9	1	3	1	
B4	14	3	#6	21	10	EB2 CAP STIRRUP	7	9	2	8				
B5	14	31	#6	22	8	EB2 CAP STIRRUP	7	9	3	1				
B6	STR	5	#5	16	8	WING C HORIZONTAL								
B7	STR	5	#5	18	3	WING C HORIZONTAL								
B8	2	3	#6	14	0	WING C STIRRUP	5	8	2	8				\perp
B9	2	16	#6	14	5	WING C STIRRUP	5	8	3	1				
B10	STR	6	#5	3	1	EB2 CAP WING HORIZ								
B11	2	4	#6	11	5	EB2 CAP WING STIRRUP	4	2	3	1				
B12	8	16	#6	18	6	WING D HORIZONTAL	16	9	1	9	1	3	1	
B13	8	2	#5	17	11	WING D HORIZONTAL	16	2	1	9	1	3	1	
B14	8	2	#5	13	7	WING D HORIZONTAL	11	10	1	9	1	3	1	
B15	8	2	#5	9	3	WING D HORIZONTAL	7	6	1	9	1	3	1	
B16	8	2	#5	18	9	WING D HORIZONTAL	17	0	1	9	1	3	1	
B17	STR	2	#5	10	3	WING D VERTICAL								
B18	STR	2	#5	10	1	WING D VERTICAL								
B19	STR	2	#5	9	10	WING D VERTICAL								П
B20	STR	2	#5	9	8	WING D VERTICAL								
B21	STR	2	#5	9	5	WING D VERTICAL								
B22	STR	2	#5	9	2	WING D VERTICAL								П
B23	STR	2	#5	8	11	WING D VERTICAL								П
B24	STR	2	#5	8	9	WING D VERTICAL	-							т
B25	STR	2	#5	8	6	WING D VERTICAL	1							т
B26	STR	2	#5	8	3	WING D VERTICAL	_							\vdash
B27	STR	2	#5	8	0	WING D VERTICAL	+						_	\vdash
B28	STR	2	#5	7	10	WING D VERTICAL	+						_	\vdash
B29	STR	2	#5	7	6	WING D VERTICAL	+						-	\vdash
B30	STR	2	#5	7	4	WING D VERTICAL	-	_			_	_	-	-



NOTE: Bar marks designated as 'e' should be epoxy coated.

FEMA Brldge: D23A 4663-DR

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

REVISION DATE

THOUGHT
COMMET

PREPARED BY
HMB PROFESSIONAL
ENGINEERS, INC.

 DATE:
 10/15/2022
 CHECKED BY
 END BENT BILL OF REINFORCEMENT

 DESIGNED BY:
 L. BOLLER
 B. REID
 CROSSING

 DETAILED BY:
 L. BOLLER
 B. REID
 HOLLYBUSH CREEK

CR-1108 TEM NO. 243.0TH COUNTY OF CO

General Notes

SPECIFICATIONS: All references to the standard Specifications are to the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, with current supplemental specifications. All references to the AASHTO are to the current edition of the AASHTO LRFD Bridge Design Specifications,

DESIGN LOADS: Beam & Slab sections are designed for 1.25*HL93 (KYHL93) Live Load.

DESIGN LOAD DISTRIBUTION : Beams are designed according to the AASHTO LRFD Bridge Design Specifications for beam spacings up to 6ft and overhangs up to 3'-0" with a 9" wide barrier.

FUTURE WEARING SURFACE : These beams are designed for a 15 PSF future wearing surface

SUBSTRUCTURE DESIGN LOADS : Unfactored design reaction forces per beam end.

DC (kips): Beam, Slab, Diaphragms, SIP Forms, and assumed railing dead loads of 533 lbs/ft. DW (kips): Future wearing surface.

LL+I (klps): LL with Dynamic load allowance.

MATERIAL DESIGN SPECIFICATIONS:

for Beam Steel FY = 50000 PSI for Steel ReInforcement for Class "AA" Deck Concrete = 60000 PS = 4000 PSI

MATERIAL STEEL A.S.T.M AASHTO

High Strength Low Alloy A709 GR 50 M270 GR 50

Structural Steel

Shear Stud Connectors UNS G 1018 M-169

Sheet lead and Pig Lead B29-79

High strength bolts, nuts, and washers F3125 Grade A325 M-164 Type 1

All steel in longitudinal rolled wide flange beams shall meet the longitudinal Charpy V-Notch toughness test for non-fracture critical components Zone 2 in accordance with the following:

M270 GR 50 (up to 2" thickness) of 15 ft-lbs at 40°F.

Sampling and testing procedures shall be in accordance with AASHTO T243 current edition, utilizing (H) frequency testing. When plate thickness exceeds 1 ½" frequency of testing shall be (P).

HIGH STRENGTH BOLT CONNECTIONS : Unless otherwise specified on the plans, all bolted connections shall be ASTM F3125 Grade A325 3/4 diameter high strength bolts, nuts, and washers. Open holes shall be ¹³/₁₆ diameter. Type 1 galvanized bolts shall be used as described in AASHTO M164. All high strength bolted field connections are to be installed with "direct tension indicators" (DTI's) in accordance with the Standard Specifications and ASTM F959. All DTI's shall be manufactured from a steel conforming to the chemical requirements of

ASTM A325 for Type 1 galvanized steel. DTI's shall be installed under the bolt head with the bumps facing the underside of the bolt head. Put a hardened washer under the nut and tension from the nut.

BEVELED EDGES : Bevel all exposed edges 3/4".

REINFORCEMENT: Dimensions shown from the face of concrete to reinforcement are to center of bars unless otherwise shown. Spacing of bars is from center to center of bars. Clear distance to face of concrete is 2" unless otherwise noted. Epoxy coat all bars. Use stirrup bend diameters for all bent bars.

CORROSION PROTECTION: These beams and all steel components are to be hot dip galvanized according to ASTM A123. Weathering Steel is not allowed.

SHEAR CONNECTORS: The minimum length of studs is 6". Provide the necessary length to penetrate at least 2" above bottom of slab.

Include all costs for shear connectors with the price of the steel beams. Including shear connectors, welding and welding material, and materials necessary to field weld or shop weld the shear connectors in place according to the plans and specifications.

If the Contractor wishes to use something other than the stud shear connectors shown on the plans, the proposed arrangement shall be submitted for approval with the shop plans.

Studs shall be welded in accordance with AWS Specifications.

MILL TEST REPORTS: Notarized mill test reports shall be furnished in triplicate to the Department, showing that all material used in the structural steel conform to the requirements of the specifications.

PROHIBITED WELDING: No welding of any nature, other than indicated on the plans, is to be performed without the written consent of the designer, and then only in the manner and at the locations designated in the authorization.

SLAB: Ensure the entire superstructure slab and diaphragms are poured continuously, out to out, before allowing any concrete to set.

SHOP DRAWINGS: The fabricator shall submit all required shop plans, by email, to the design engineer for review. These submissions shall depict the shop plans in .pdf format. As either 11"x17" or 22"x36" sheets. Designers will make review comments on these electronic submissions as needed and shall return them to the fabricator. Upon reconciliation of the designers comments, files shall be returned to the designer and plans will be forwarded to the Division of Structural Designs Shop Plan coordinator for distribution. Only plans submitted directly to the shop plan coordinator will be distributed and only plans electronically stamped "Distributed by The Division of Structural Design" are to be used for fabrication. While this process does not require the submission of paper copies, The Division of Structural Design reserves the right to require such copies on a case by case basis.

When any changes to the design plans are proposed by the Fabricator or Supplier, the shop drawings reflecting these changes shall be submitted to the Design Engineer contractor

This plan sheet has been adapted from Standard Drawing BSB-101. Shaded regions on this plan sheet do not apply to this project.

FEMA Bridge: D23A 4663-DR

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

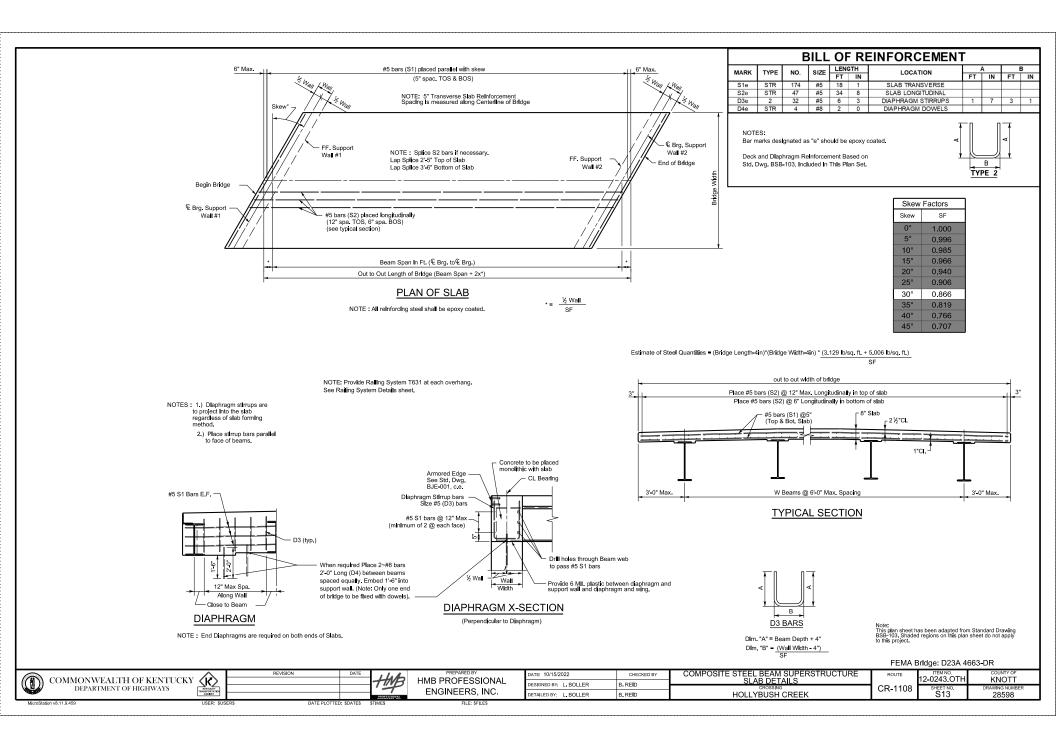
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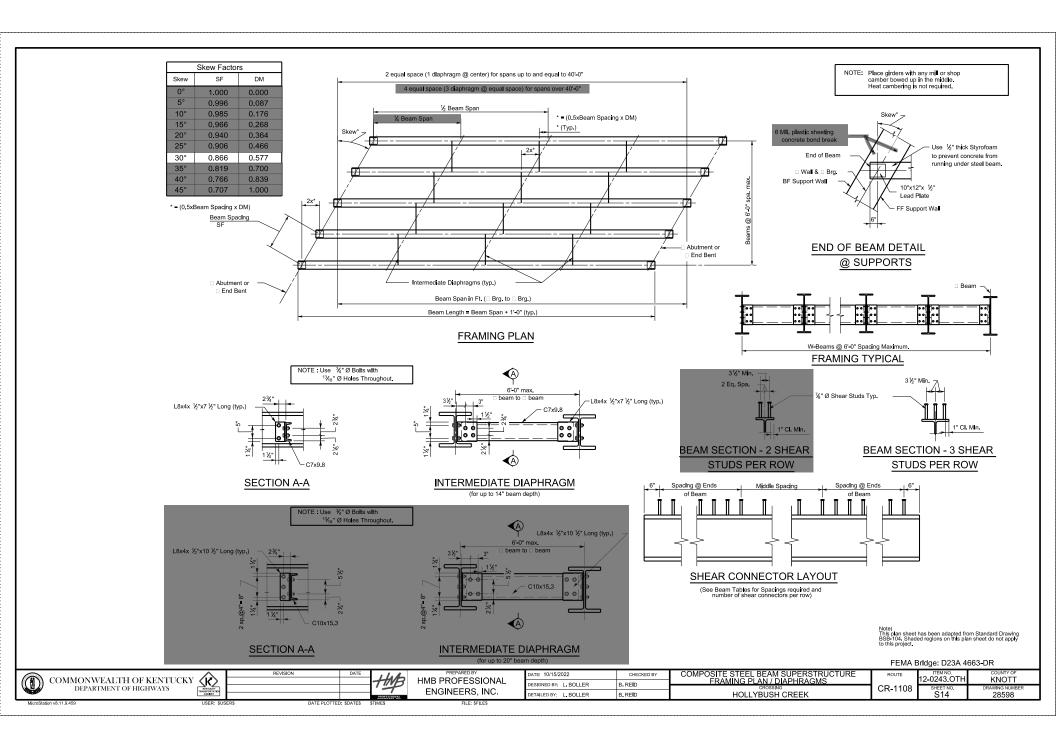
HMB PROFESSIONAL ENGINEERS, INC.

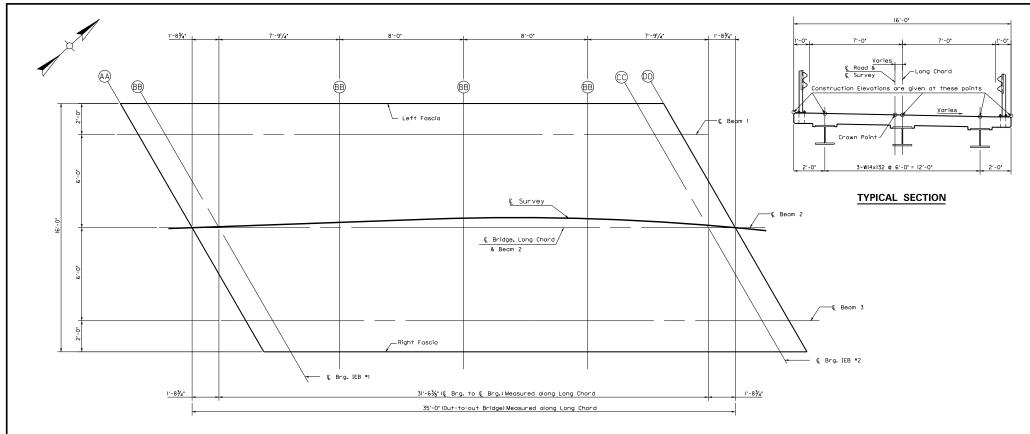
COMPOSITE STEEL BEAM SUPERSTRUCTURE DATE: 10/15/2022 CHECKED BY **GENERAL NOTES** DESIGNED BY: L. BOLLER . REID DETAILED BY: L. BOLLER B, REID HOLLYBUSH CREEK

2-0243 OT KNOTT CR-1108 SHEET NO S11 28598

BEAM	ROLLED) BEAM	DEFLECTION	IN INCHES		SHEAR CONNI	ECTORS		UNFACTOR	ED BEAM END F	REACTION	LL DIS	ST. FACT. ANES)		
SPAN	BEAM MEMBER	BEAM DEPTH (IN.)	STEEL ONLY	TOTAL DL	#Per Row	Spa. @ Ea. End of Beam	Middle Spacing	ADTT	DC (kips)	DW (kips)	LL+I (kips)	LLDFM	LLDFV		
0' Max.	W18x55	18.1	0.00	0.11	2	10 Spa. @ 5 1/4"	5¾" Spa.	300	11.85	0.90	72.65		.877		
	W16x77	16.5	0.00	0.09	3	5 Spa. @ 5 ¾"	6" Spa.	Inf.	12.10		71.61		.865		
	W14x82	14.3	0.00	0.11	3	14 Spa. @ 5 1/4"	6" Spa.	450	12.14		72.36		.874		
	W12x106	12.9	0.00	0.11	3	14 Spa. @ 5 1/4"	6" Spa.	400	12.40		71.78		.867		
25' Max.	W24x62	23.7	0.02	0.17	2	23 Spa. @ 5 1/4"	6" Spa.	400	14.75	1.13	74.47		.860		
	W21x62	21.0	0.03	0.21	2	23 Spa. @ 5 1/4"	6" Spa.	350	14.75		75.31		.870		
	W18x71	18.5	0.03	0.24	2	23 Spa. @ 5 1/4"	6" Spa.	290	14.86	4	75.67		.874		
	W16x77	16.5	0.04	0.25	3	11 Spa. @ 5 ½"	6" Spa.	Inf.	14.96		76.05		.878		
	W14x82	14.3	0.05	0.31	3	23 Spa. @ 5 1/4"	6" Spa.	425	15.01	_	76.89	-	.888		
	W12x106	12.9	0.05	0.31	3	23 Spa. @ 5 1/4"	6" Spa.	385	15.33	1.05	76.25	-	.880		
30' Max.	W24x76	23.9	0.04	0.28	2	28 Spa. @ 5 1/4"	6" Spa.	415	17.78	1.35	80.90	1	.856		
	W21x83	21.4	0.05	0.32	2 3	28 Spa. @ 5 1/4"	6" Spa.	350	17.89		81.41		.862		
	W18x86	18.4	0.06	0.39	3	7 Spa. @ 5 ¾"	6" Spa.	Inf.	17.94		82.40		.872		
	W16x100	17.0	0.06	0.40	3	14 Spa. @ 5 ½" 28 Spa. @ 5 ½"	6" Spa.	Inf. 440	18.18		82.26		.871		
	W14x120 W12x120	14.5	0.08	0.45	3	, ,	6" Spa.	375	18.48 18.48		82.48 83.41		.883		
35' Max.	W12x120 W27x84	13.1 26.7	0.10	0.57	3	28 Spa. @ 5 1/4"	6" Spa.	3/5 Inf.	18.48 20.79	1.58	86.49		.850		
JJ WAX.	W24x94	24.3	0.06	0.39	2	6 Spa. @ 7 ¾" 24 Spa. @ 5 ¼"	8" Spa. 6" Spa.	420	20.79	1.56	86.69	1	.850		
	W21x101	21.4	0.00	0.47	3	7 Spa. @ 6 ½"	7" Spa.	Inf.	21.10	-	87.33	1	.858		
	W18x119	19.0	0.07	0.47	3	7 Spa. @ 6"	6½" Spa.	Inf.	21.10	-	87.50	1	.860		
	W14x132	14.7	0.09	0.76	3	24 Spa. @ 5 1/4"	6" Spa.	425	21.43		89.15		.876	USE W14×132	
	W12x152	13.7	0.17	0.82	3	24 Spa. @ 5 ¾"	6" Spa.	375	22.03		89.00		.874	036 1114 1132	
40' Max.	W30x99	29.7	0.07	0.48	3	12 Spa. @ 8"	9" Spa.	Inf.	23.97	1.80	90.47	1	.840		
TO MIGA.	W27x102	27.1	0.08	0.53	3	20 Spa. @ 7 ½"	9" Spa.	Inf.	24.03	- 1.00	91.07	1	.845		
	W24x117	24.3	0.09	0.56	3	14 Spa. @ 7"	8" Spa.	Inf.	24.35	-	91.22	1	.846		
	W21x122	21.7	0.12	0.67	3	16 Spa. @ 6"	7" Spa.	Inf.	24.45		92.04	0.65	.854		
	W18x130	19.3	0.15	0.81	3	9 Spa. @ 5 ½"	6" Spa.	Inf.	24.62		92.79	0.65	.861		
	W14x176	15.2	0.21	0.98	3	19 Spa. @ 5 1/4"	6" Spa.	425	25.58		93.11		.864		
	W12x190	14.4	0.25	1.12	3	19 Spa. @ 5 1/4"	6" Spa.	375	25.86		93.25	1	.865		
45' Max.	W33x118	32.9	0.09	0.54	3	12 Spa. @ 9"	10" Spa.	Inf.	27.33	2.03	93.45	1	.828		
	W30x116	30.0	0.11	0.64	3	21 Spa. @ 8"	10" Spa.	Inf.	27.29		94.36	1	.836		
	W27x129	27.6	0.12	0.68	3	15 Spa. @ 7 ½"	9" Spa.	Inf.	27.57		94.53	1	.838		
	W24x131	24.5	0.14	0.80	3	16 Spa. @ 7"	8" Spa.	Inf.	27.63		95.48		.846		
	W21x147	22.1	0.17	0.91	3	17 Spa. @ 6 ½"	7" Spa.	Inf.	28.00		95.83		.849		
	W18x158	19.7	0.22	1.08	3	10 Spa @ 5 ½"	6" Spa.	Inf.	28.25		96.54		.855		
50' Max.	W33x130	33.1	0.13	0.74	3	23 Spa. @ 8"	10" Spa.	Inf.	30.60	2.25	97.11		.828		
	W30x132	30.3	0.15	0.86	3	24 Spa. @ 7 1/2"	9" Spa.	Inf.	30.65		97.86		.834		
	W27x146	27.4	0.17	0.89	3	18 Spa. @ 7"	8" Spa.	Inf.	31.02		98.11		.836		
	W24x162	25.0	0.20	0.99	3	28 Spa. @ 6 1/2"	8" Spa.	Inf.	31.43		98.43		.839		
	W21x182	22.7	0.24	1.11	3	20 Spa. @ 6"	7" Spa.	Inf.	31.95		98.70		.841		
	W18x192	20.4	0.30	1.36	3	35 Spa. @ 5 1/4"	7" Spa.	Inf.	32.19		99.51		.848		
55' Max.	W36x135	35.6	0.17	0.93	3	16 Spa. @ 8 1/4"	10" Spa.	Inf.	33.73	2.48	100.10		.825		
	W33x141	33.3	0.18	0.99	3	25 Spa. @ 8"	10" Spa.	Inf.	33.91		100.45		.828		
	W30x148	30.7	0.21	1.11	3	28 Spa. @ 7 1/4"	9" Spa.	Inf.	34.09		100.97		.832		
	W27x178	27.8	0.23	1.10	3	19 Spa. @ 7"	8" Spa.	Inf.	34.96		100.75		.830		
	W24x192	25.5	0.27	1.25	3	31 Spa. @ 6 1/2"	8" Spa.	Inf.	35.33		101.20		.834		
	W21x223	23.4	0.33	1.35	3	33 Spa. @ 6"	8" Spa.	Inf.	36.21		101.10		.833		Note:
	W18x258	21.5	0.40	1.51	3	35 Spa. @ 5 ¾"	7" Spa.	Inf.	37.22		101.21		.834		This plan sheet has been adapted from Standard BSB-102. Shaded regions on this plan sheet do
60' Max.	W36x150	35.9	0.22	1.17	3	27 Spa. @ 8 1/4"	10" Spa.	Inf.	37.20	2.70	102.92		.823		to this project.
	W33x169	33.8	0.24	1.16	3	18 Spa. @ 8"	9" Spa.	Inf.	37.76		102.82		.822		EEMA Deldo DOOA 4000 DD
				REVISI	ON	DATE		PREPARED BY		DATE: 10/15/2022		CHECKED	DEV 1	COMPOSITE STEEL BEAM SUPERSTRUCTURE	FEMA Bridge: D23A 4663-DR
COI	MMONWEAL		icky 👠 🗀	ILCVISI		HATE HA	В НМВ Р	ROFES	SIONAL	DESIGNED BY: L. BC	LLER F	CHECKED B. REID	,,,,	BEAM TABLES	12-0243.OTH KN
-	DEPARTMENT	OF HIGHWAYS	RENTUCKY TRANSPORTATION CASSNET					INEERS	, INC.	DETAILED BY: L. BC		B, REID		HOLLYBUSH CREEK	CR-1108 SHEET NO. DRAWIN S12 28







NOTES FOR ELEVATIONS TAKEN ON STEEL BEAMS

Take elevations on top of beam at points indicated by the grid layout. The beam elevations are to be read to three decimals, and entered in tables under 'Top of Beam' elevations.

Compute dimension 'X' as follows: 'Construction Elevation' minus 'Top of Beam' elevation equals dimension 'X'. Construction Elevations include comber due to weight of the concrete slab and barrier. Measuring of dimension "X" gives the final check on beam tolerances for camber, beam damage, and errors in erection that produce reverse cambers, sags, and unsightly fascia beams.

For setting templates, measure dimension "X" above top of beams for top of template. Do not set template by elevations.

Temporary supports or shoring will not be permitted under the girders when pouring the concrete floor slab or when taking 'Top of Beam' elevations.

Construct rail to roadway grade. Do not add camber to the rail.

CONSTRUCTION ELEVATIONS

LOCATION	LEET	BEAM 1			BEAM 2 & LONG CHORD				RIGHT		
LUCATION	LEFT FASCIA	CONSTR.	TOP OF	DIM.	CONSTR.	TOP OF	DIM.	CONSTR.	TOP OF	DIM.	RICHT FASCIA
		ELEV.	BEAM	.x.	ELEV.	BEAM	.x.	ELEV.	BEAM	•x•	
SKEW LN AA	895.753	895.785			895.864			895.917			895.929
SKEW LN BB	895.776	895.806			895.875			895.920			895.929
SKEW LN CC	895.925	895.907			895.832			895.726			895.684
SKEW LN DD	895.921	895.900			895.818			895.704			895.658
GRID LN 01	896.285	896.289			896.238			896.186			896.191
GRID LN 02	896.338	896.335			896.308			896.325			896.322
GRID LN 03	896.148	896.137			896.132			896.216			896.204

FEMA Brldge: D23A 4663-DR

DATE: 10/15/2022 CHECKED BY CONSTRUCTION ELEVATIONS COMMONWEALTH OF KENTUCKY HMB PROFESSIONAL 2-0243 OTI KNOTT DESIGNED BY: L. BOLLER B. REID DEPARTMENT OF HIGHWAYS CR-1108 WING NUMBE 28598 ENGINEERS, INC. SHEET NO S15 RENTUCKY TRANSPORTATION CASINET DETAILED BY: L. BOLLER B, REID HOLLYBUSH CREEK

COUNTY OF	ITEM NO.	SHEET NO.
KNOTT	12-0243.0TH	U1

UTILITY SUMMARY

ITEM	DESCRIPTION	UNIT	SHEET U3			PROJECT TOTALS
WATER	LINE SUMMARY					
14000	W AIR RELEASE VALVE 1 INCH	EA	1			1
14003	W CAP EXISTING MAIN	EA	2			2
14004	W DIRECTIONAL BORE	LF	160			160
14059	W PIPE PVC 06	LF	86			86
14089	W TAPPING SLEEVE AND VALVE SIZE 1	EA	2			2
21233ED	ASPHALT PAVING REPLACEMENT	LF	85			85
				1	l	

UTILITY CONSTRUCTION TO BE COMPLETED BY OTHER PRIOR TO CONSTRUCTION. UTILITY PLANS ARE FOR INFORMATION ONLY.

UTILITY SUMMARY HOLLYBUSH ROAD AT HOLLYBUSH CREEK

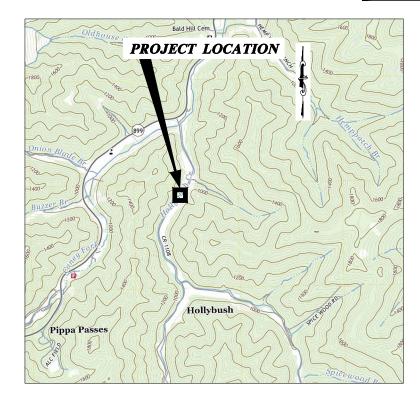
COUNTY OF	ITEM NO.	SHEET NO.
KNOTT	12-0243.0TH	U2

GENERAL NOTES

- PRIOR TO BEGINNING CONSTRUCTION, THE CONTRACTOR IS TO VERIFY ALL FINISHED GRADES AND DIMENSIONS IN THE FIELD AND REPORT ANY DISCREPANCIES TO THE ENGINEER.
- DIMENSIONS OF EXIST, STRUCTURES AND/OR SIZE RESTRICTIONS ARE APPROXIMATE. ALL NECESSARY DIMENSIONS AND ELEVATIONS OF EXIST, STRUCTURES 4. TOPOGRAPHY SHALL BE VERFIED BY THE CONTRACTOR IN THE FIELD PRIOR TO HIS
- THE CONTRACTOR SHALL OBTAIN REQUIRED PERMITS AND GIVE ALL NOTICES REQUIRED FOR EXECUTION OF THE WORK. THE CONTRACTOR SHALL PAY FOR ALL PERMITS AND ANY APPLICABLE SERVICES INSPECTION FEES.
- THE CONTRACTOR'S VORK AND OPERATIONS SHALL CONFORM TO ALL LOCAL, STATE AND FEDERAL CODES AND RECEIVE APPROVAL WHERE NECESSARY PRIOR TO COMMENCEMENT OF THE VORK.
- ALL MATERIALS BEING REMOVED AND NOT RE-USED IN THE NEW CONSTRUCTION SHALL BE FIRST DIFFERED TO THE OWNER. ALL MATERIALS NOT ACCEPTED SHALL BE PROPERLY DISPOSED OF BY THE CONTRACTOR.
- ALL BURIED PIPES SHALL HAVE A MINIMUM OF 3'-6" COVER AS HEASURED VERTICALLY FROM FINISHED GRADE TO THE TOP OF PIPE, UNLESS OTHERVISE NOTED.

- IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VIDER ALL APPLICABLE BRANNESS AND SPECIFICATIONS AS A UNIT. ANY ORISSIDES, BELETIONS AND/ESCOPILITY AS RESPONSED BY THE CONFICT AND ASSECTION OF ANY ORISSIDES, BELL BRANNESS AND SPECIFICATIONS WHICH APPLY SHALL BE CORRECTED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE CUMPS.

- BOLD STRENGTH LINES REPRESENT PROPOSED WORK, LIGHT/SHADED LINES REPRESENT EXIST. FACILITIES.
- THE ENGINEER RESERVES THE RIGHT TO MAKE MINDR ADJUSTMENTS IN THE WORK IN DRDER TO ACCOMPLISH THE INTENT OF THE DESIGN
- GENERALLY SLOPE GRADES UNIFORMLY BETWEEN NOTED GRADE POINTS.
- ALL EARTH FILL TO BE COMPACTED IN ACCORDANCE WITH THE SPECIFICATIONS.
- CONTRACTOR SHALL CLEAN SITE/ROADS WEEKLY, UNLESS REQUESTED MORE FREQUENTLY.
- CONTRACTOR SHALL COORDINATE ANY TIE-INS/SHUT DOWNS WITH OWNERS PERSONNEL WITH A MINIMUM NOTIFICATION OF 7 DAYS.
- ALL PIPE JOINTS AND FITTINGS BELOW STRUCTURES SHALL BE RESTRAINED JOINTS.
- CDATING SYSTEMS REQUIRED ON ALL EXPOSED STEEL, DIP VALVES, FITTINGS AND MISC. METALS PER SECTION 09900 OF THE SPECIFICATIONS.
- ALL FLANGED PIPE/FITTING CONNECTIONS SHALL BE PROVIDED WITH TYPE 304 STAINLESS STEEL NUTS AND BOLTS/HARDWARE.
- ALL DUCTILE IRON PIPE AND FITTINGS FOR AIR PIPING SHALL BE UNLINED INCLUDING CEMENT LINING.
- ALL ANCHOR BOLTS AND HARDWARE SHALL BE TYPE 304 STAINLESS STEEL UNLESS OTHERWISE INDICATED.
- INSTALL VALVE BOXES, TRACER WIRE, AND LINE MARKERS PER SPECIFICATIONS.



LOCATION MAP SCALE : N.T.S.

BEFORE YOU DIG

The contractor is instructed to call 1-800-752-6007 to reach KY 811, the one-call system for information on the location of existing underground utilities. The call is to be placed minimum of two (2) and no more than the (12) business ddgs pitch to excavation. The minimum of two (2) and the more than the (12) business ddgs pitch to excavation. The members of the KY 81.1 one-call Before-U-Dig (8UD) service. The contractor must coordinat excavation with the utility owners, including those whom do not subscribe to KY 811. It may be necessary for the contractor to contact the County Court Clerk to determine what talkly companies have facilities in the area.

LEGEND:

EXISTING:

PROPOSED:

WATER

CONTACT INFORMATION

WATER KNOTT CO WATER & SEWER DISTRICT JARED SALMONS, GENERAL MANAGER 777 BIG BRANCH ROAD VICCO, KY, 41773 (606) 645-3582

INDEX OF SHEETS

KYTC UTILITY SUMMARY

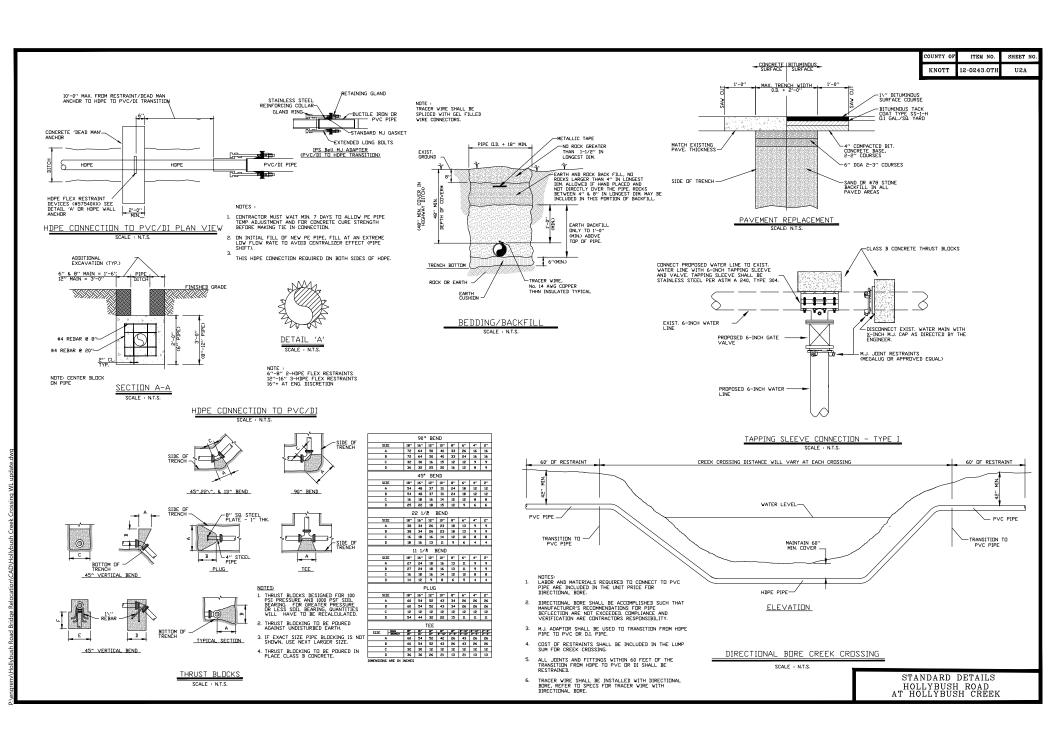
GENERAL NOTES, LOCATION MAP & INDEX OF SHEETS

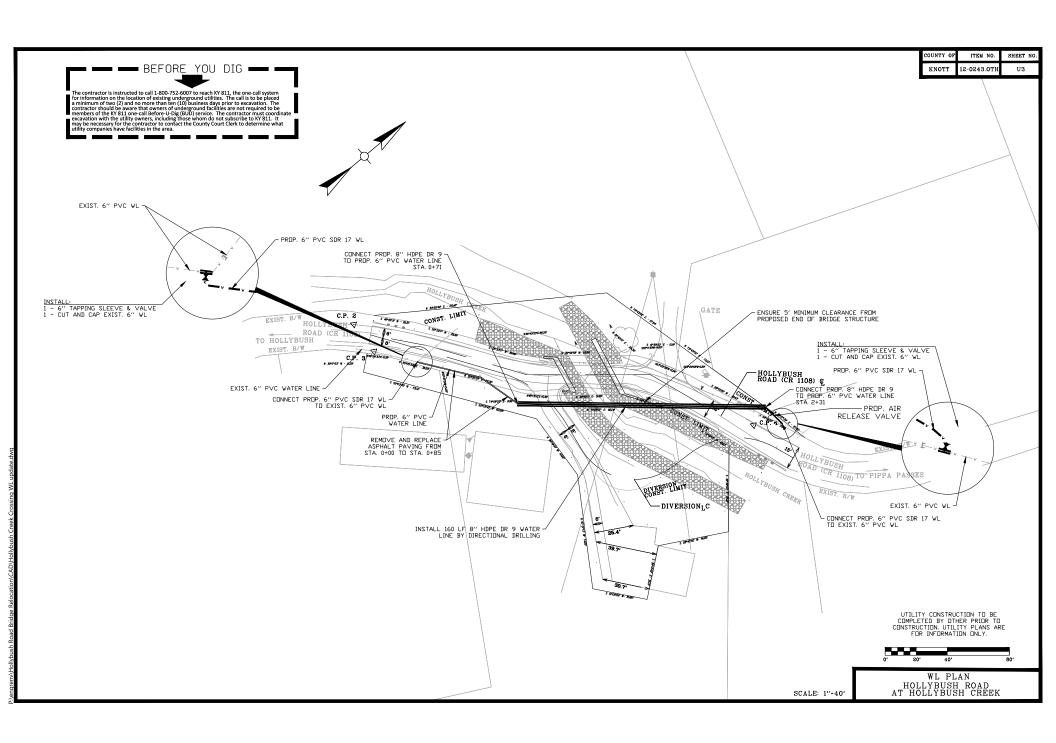
LI2A STANDARD DETAILS U3

HOLLYBUSH CREEK WATER LINE - PLAN HOLLYBUSH CREEK WATER LINE - PROFILE

UTILITY CONSTRUCTION TO BE COMPLETED BY OTHER PRIOR TO CONSTRUCTION. UTILITY PLANS ARE FOR INFORMATION ONLY.

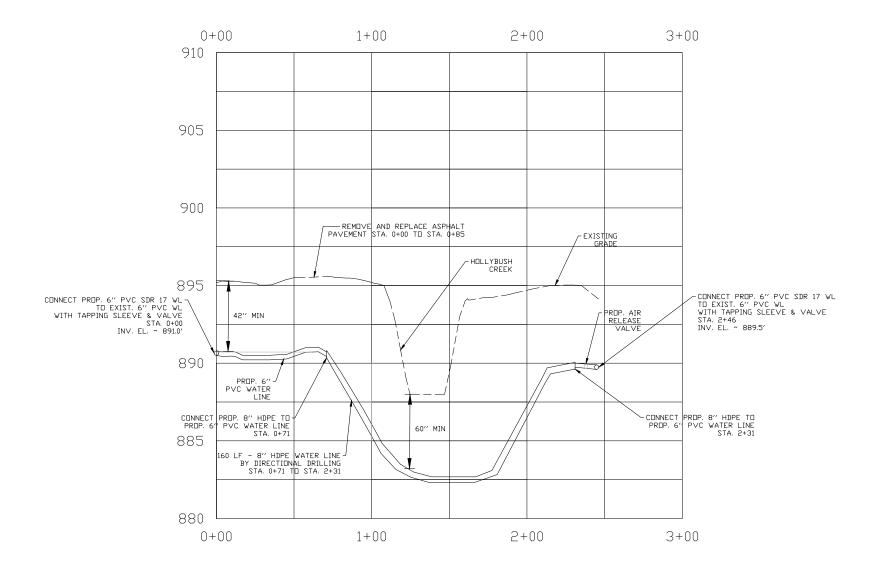
NOTES, INDEX AND MAP HOLLYBUSH ROAD AT HOLLYBUSH CREEK



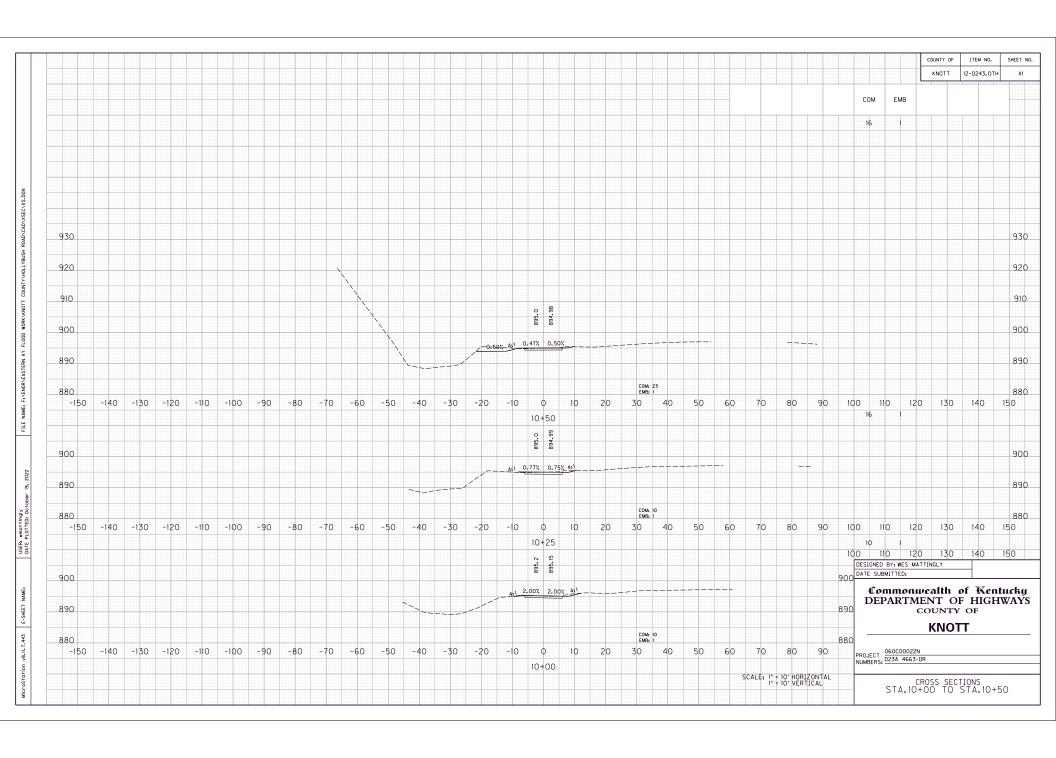


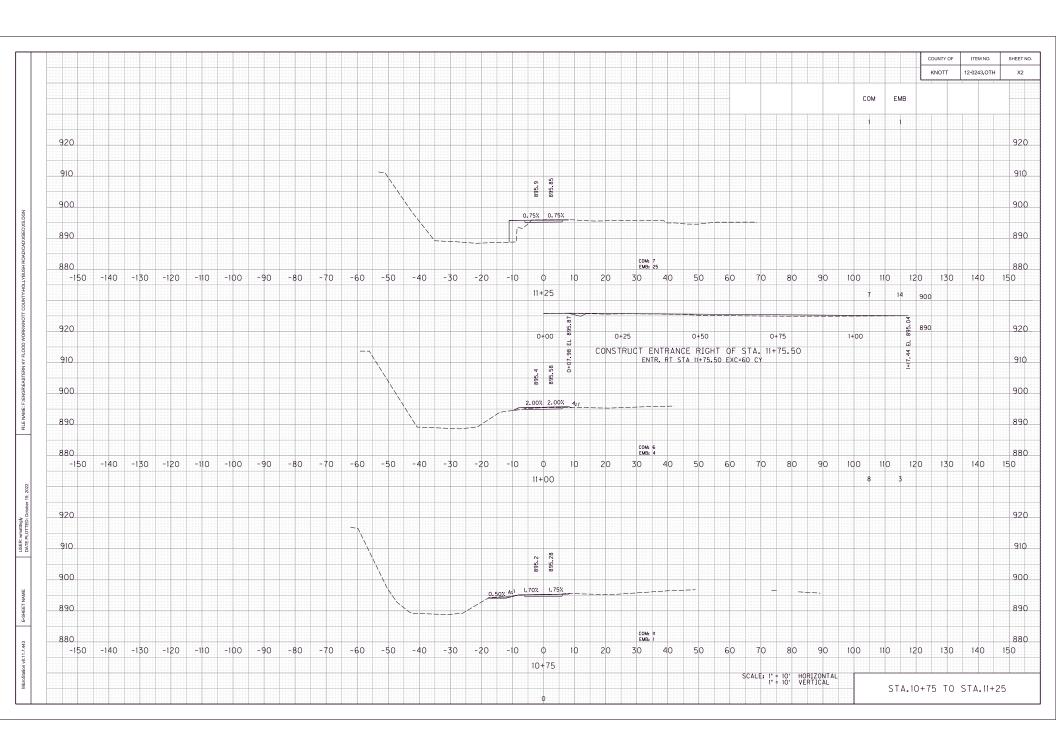
COUNTY OF ITEM NO. SHEET NO.

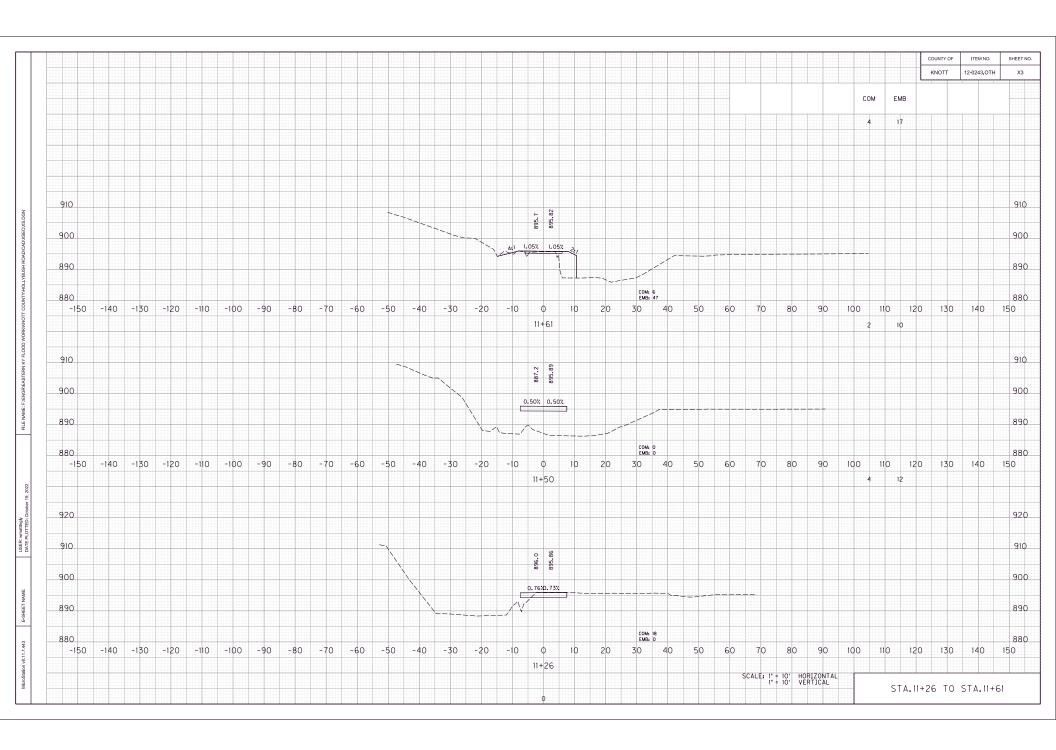
KNOTT 12-0243.0TH U4

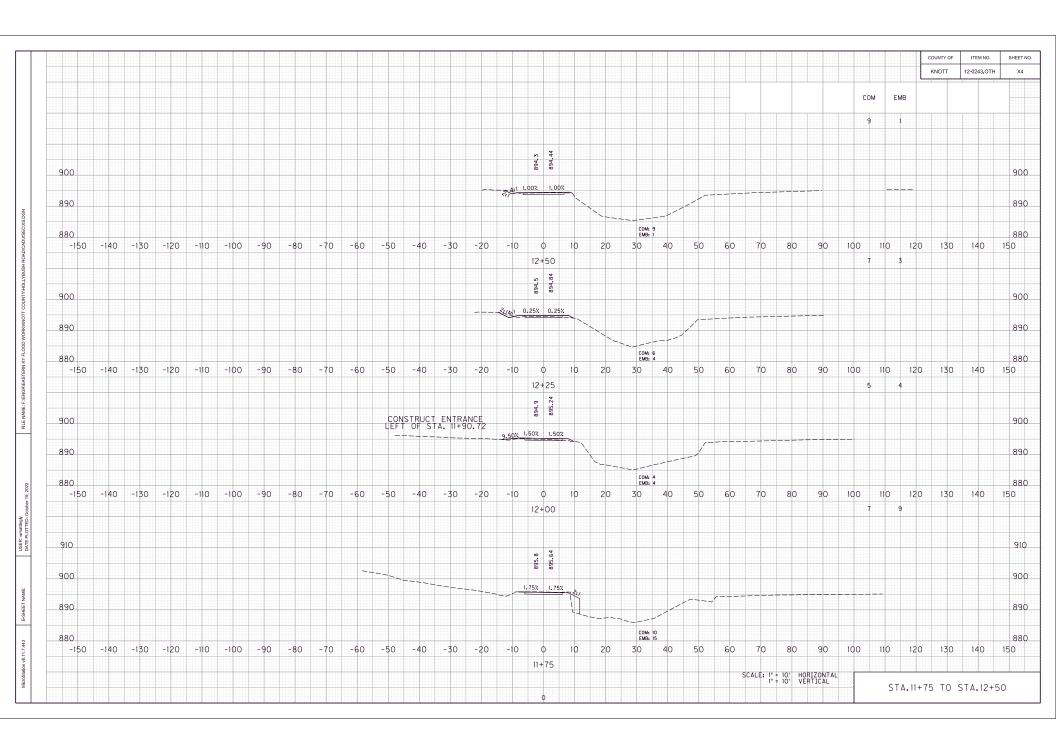


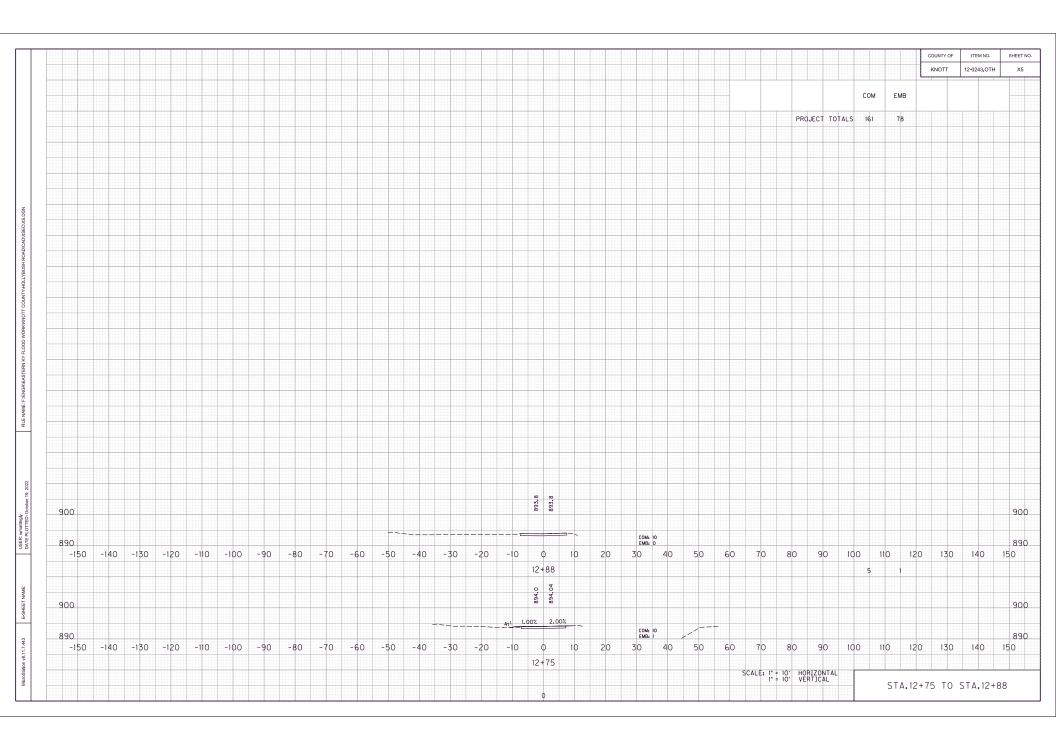
SCALE: 1"-4' VERT. 1"-40' HORIZ. WL PROFILE HOLLYBUSH ROAD AT HOLLYBUSH CREEK











		STANDARD DRAWINGS								
	BDP-001-06	BOX BEAM GENERAL NOTES AND REFERENCES								
	BDP-002-03	BOX BEAM BEARING DETAILS								
	BDP-003-03	BOX BEAM MISCELLANEOUS DETAILS								
	BDP-004-04	BOX BEAM TENSION ROD DETAILS								
	BDP-008-04 BOX BEAM CB21 DETAILS									
	BGX-006-10 BHS-011	RAILING SYSTEM SIDE MOUNTED MGS DETAILS								
	BJE-001-14	ARMORED EDGES								
	BPS-011-04	HP 14X89 STEEL PILE								
	0.0.0									
	RBR-010-06	GUARDRAIL TERMINAL SECTIONS								
	RBR-055-01	DELINEATORS FOR GUARDRAIL								
	RD1-040-01 RDX-210-03	EROSION CONTROL BLANKET SLOPE INSTALLATION TEMPORARY SILT FENCE								
	RDX-220-05	SILT TRAP TYPE A								
	RDX-225-01	SILT TRAP TYPE B								
	RDX-230-1	SILT TRAP TYPE C								
	RGX-001-06	MISCELLANEOUS STANDARDS								
	RGX-100-07	TREATMENT OF EMBANKMENTS AT END-BENTS								
	RGX-105-09 RPM-110-07	TREATMENT OF EMBANKMENTS AT END-BENTS - DETAILS								
	TTC150-04	APPROACHES, ENTRANCES AND MAIL BOX TURNOUT ROAD CLOSURE WITH DIVERSON								
	TTC155-02	TEMP. PAVEMENT MARKER ARRANGEMENTS FOR CONST. ZONES								
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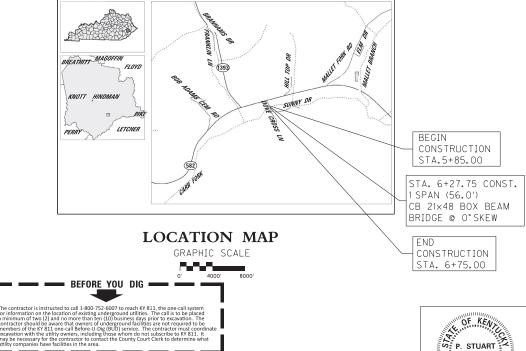
TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS **KNOTT COUNTY**

DOVE CROSS LANE (CR 1538) OVER CARR FORK FEMA BRIDGE: 4663-DR

STA. 6 + 27.75

REV. NO.

EX BRIDGE ID 060C001



SHEETS REVISED TABLE OF REVISIONS PREPARED BY

J.M. Crawford & Associates Consulting Engineers

SHEET NO. KNOTT

	INDEX OF SHEETS										
Sheet No.	Description										
RI	LAYOUT SHEET										
R2	LEGEND AND TYPICAL SECTIONS										
R3	ROADWAY PLAN SHEET										
R4	ROADWAY PROFILE SHEET										
XI-X3	CROSS SECTIONS										
SI	TITLE SHEET										
S2	GENERAL NOTES										
S3	LAYOUT										
S4	FOUNDATION LAYOUT										
S5	END BENT "I DETAILS										
S6	END BENT *2 DETAILS										
S7	SUPERSTRUCTURE										
S8	CONSTRUCTION ELEVATIONS										
	SPECIAL NOTES										

SPECIAL PROVISIONS

69 EMBANKMENT AT BRIDGE END BENT STRUCTURES

PLACING BRIDGE OVERLAY APPROACH PAVEMENT FROSION PREVENTION AND SEDIMENT CONTROL TRAFFIC CONTROL ON BRIDGE REPAIR CONTRACT

SPECIFICATIONS

2019 Standard Specifications for Road and Bridge

2020 AASHTO LRFD Bridge Design Specifications with

REVISION

Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS COUNTY OF

KNOTT

DOVE CROSS LANE OVER CARR FORK TEM NO. N/A

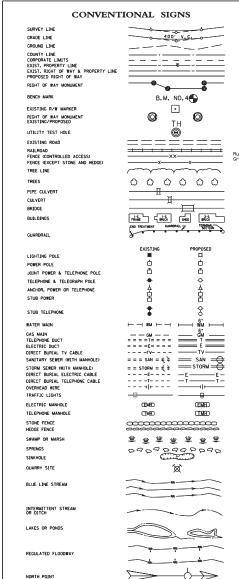
RAWING NO. _28541 ROJECT ETTING DATE: .

STATE HIGHWAY ENGINEER

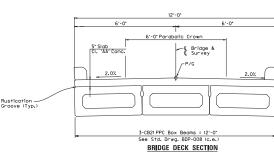
P. STUART 19302 19302 19302	
19302	D
19302 SONAL ENGINE	P
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P. Stot MUC	

Philip S.

Philip S. McIntosh McIntosh Date: 2022.12.16



TYPICAL SECTIONS



Varies 9' - 5" To 12' - 0" Varies - Profile Grade Vories NORMAL SECTION See Detail "A" Traffic Lane Pavement - I'/4" Depth Cl. 2 Asphalt Surface 0.38D PG64-22 Asphalt Surface Asphalt Base 8" Depth (4"+4") Cl. 2 Asphalt Base 1.00D PG 64-22 DGA Base 8" Depth (4"+4") Shoulders

DGA Bose

ASPHALT SEAL REQUIRED FROM OUTSIDE EDGE OF PAVEMENT TO A POINT 2'DOWN THE DITCH OR FILL SLOPE. TWO APPLICATIONS OF THE FOLLOWING: 1

Full Depth

ASPHALT SEAL COAT ASPHALT SEAL AGGREGATE

20 LBS/SY (SIZE NO. 8 OR 9M)

- GEOTEXTILE FABRIC CLASS 2 (SEPARATION SHALL BE INCIDENTAL TO DGA.
- 3 GRANULAR EMBANKMENT FOR NECESSARY WIDENING LOCATIONS AS APPROVED

NOTES: MATERIAL NEEDED FOR SHOULDERS OUTSIDE OF PAVED AREA WILL BE MEASURED AND PAID AS GRANULAR EMBANKMENT. GEOTEXTILE FABRIC CLASS II (SEPARATION) SHALL BE INCIDENTAL TO GRANULAR EMBANKMENT.

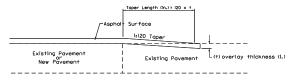
(4" + 4") Depth DGA Base (3) Detail "A"

FULL-DEPTH MAINLINE & SHOULDER PAVEMENT RECONSTRUCTION

(1)

NOT TO SCALE

TAPERING OF OVERLAYS ON LOW SPEED FACILITIES < (45mph)



EDGE KEY

Work under this item shall include cutting out the existing asphalt surface to a minimum depth and width as shown, so the new surface may heel into the existing surface. The contract unit price bid per linear foot for EDGE KEY shall include all necessary materials, labor, equipment, etc. to perform the work and dispose of the bituminous material removed.

KNOTT COUNTY BRIDGE# 060C001 DOVE CROSS LN OVER CARR FORK

Point	Description	Northing	Easting	Elevation	Station	Offset
CP #1	MAG NAIL	3636600.961	5751142.06	1096.494	4+79.43	15.54' RT
CP #2	MAG NAIL	3636372.153	5751270.82	1098.245	7+41.83	9.59'LT

Coordinates for horizontal control were established using the KY CORS Network NAD 83 using Trimble RI2's on the Kentucky Single Zone Network, US Survey Feet on August 15th, 2022, Values are expressed in US Survey Feet. No project datum factor was calculated for this project.

Basis of Elevations

The elevations were established using GPS.

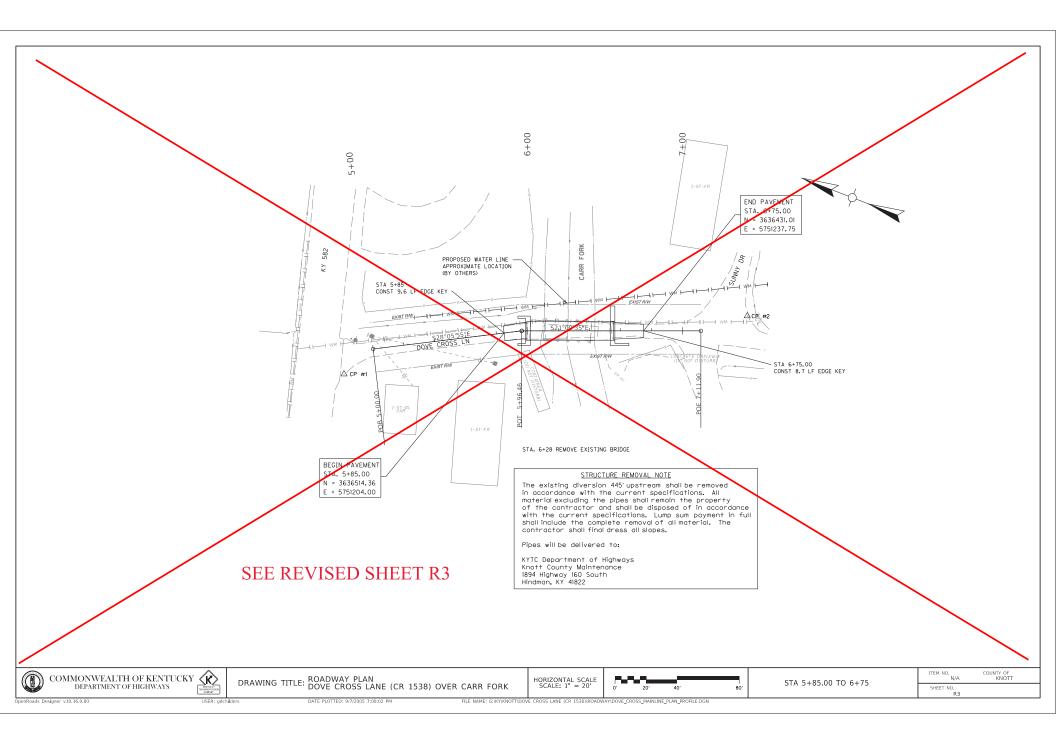
COMMONWEALTH OF KENTUCKY (K) DEPARTMENT OF HIGHWAYS

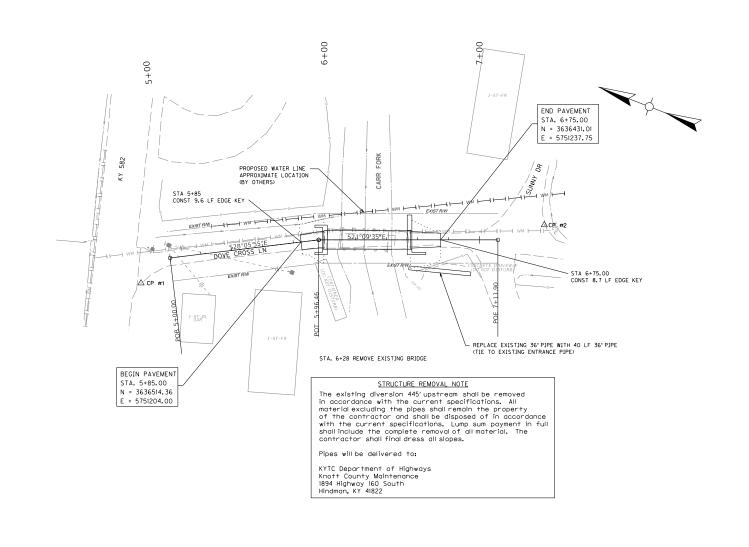
DRAWING TITLE: TYPICAL SECTIONS AND LEGEND DOVE CROSS LANE (CR 1538) OVER CARR FORK

ITEM NO. N/A COUNTY OF KNOTT SHEET NO.

11/4" Depth Ci. 2 Asphalt Surface 0.38D PG64-22

8" Depth (4"+4") Cl. 2 Asphalt Base 1.00D PG 64-22





COMMONWEALTH OF KENTUCKY ODEPARTMENT OF HIGHWAYS

DRAWING TITLE: ROADWAY PLAN
DOVE CROSS LANE (CR 1538) OVER CARR FORK

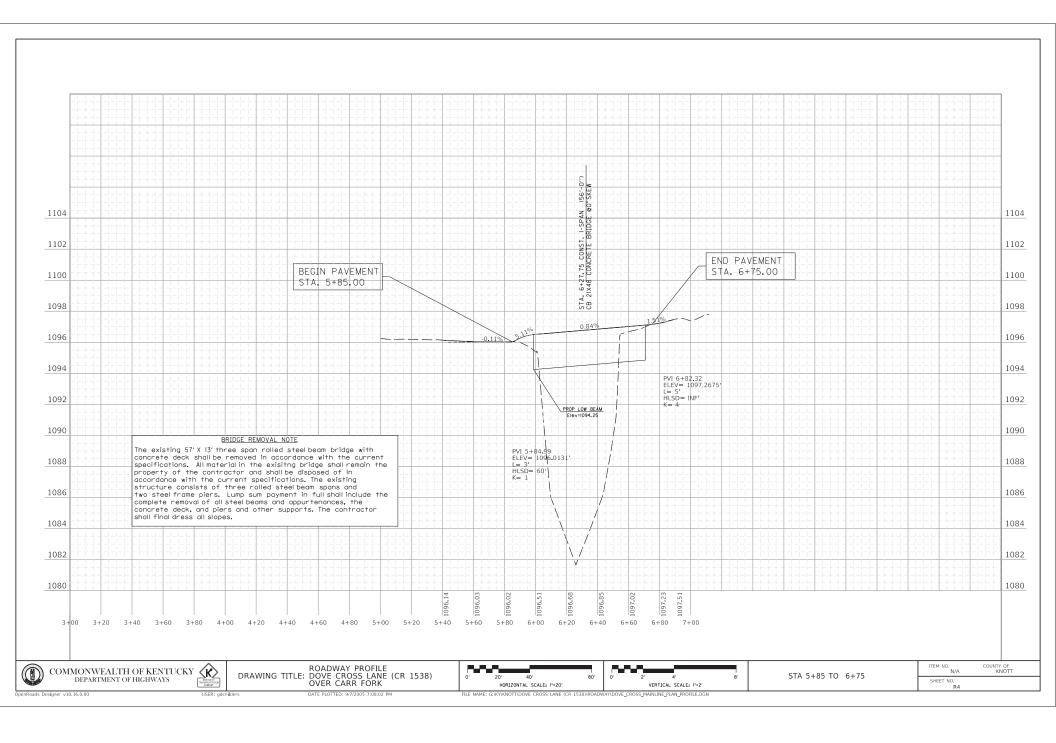
HORIZONTAL SCALE SCALE: 1" = 20'

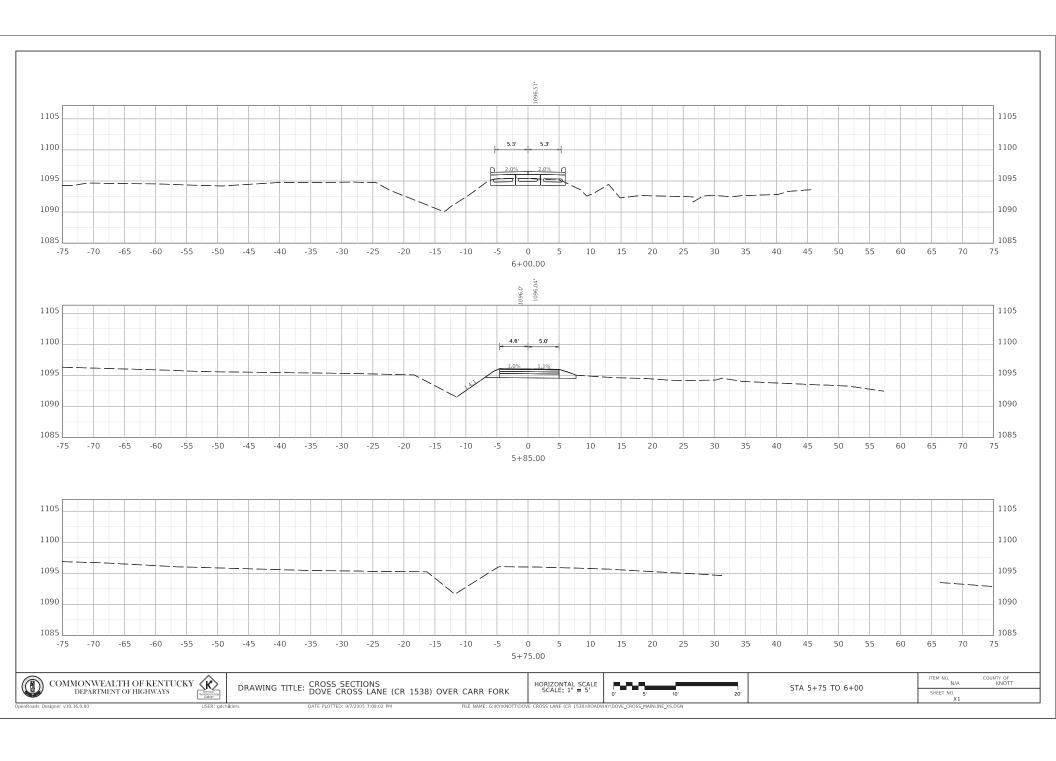


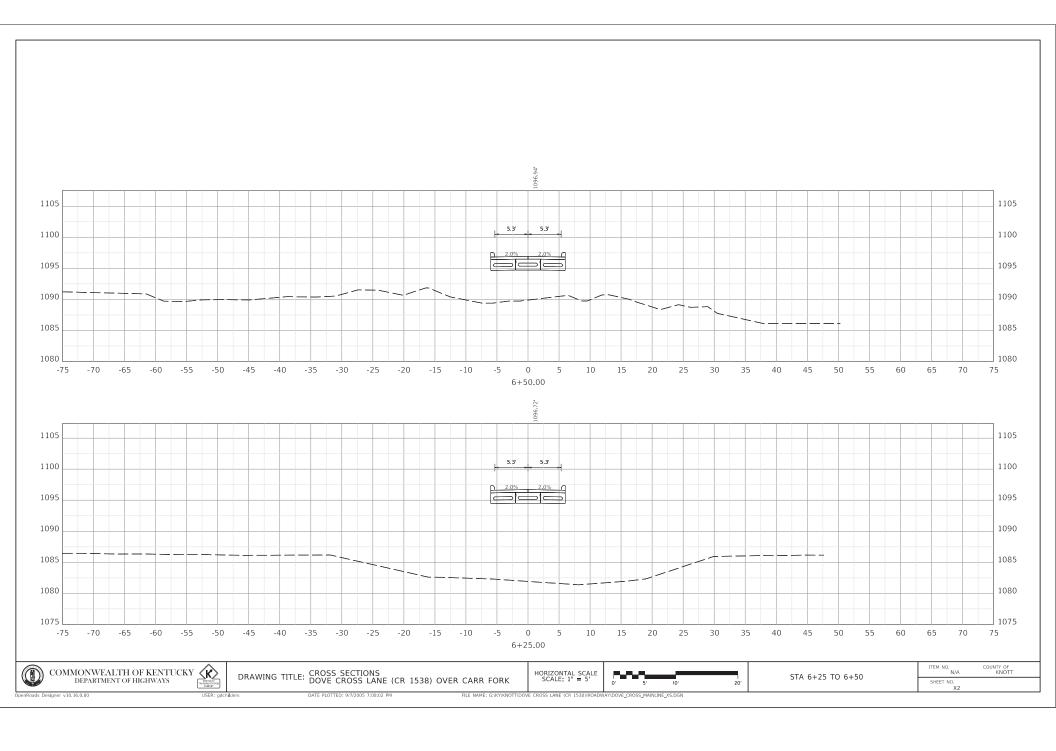
STA 5+85.00 TO 6+75

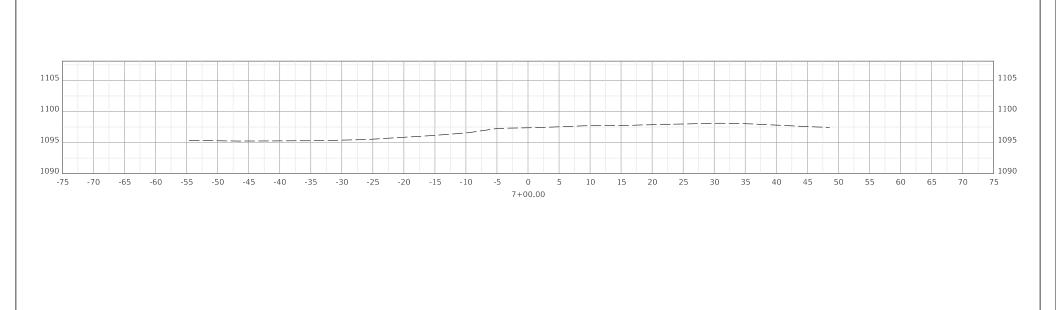
ITEM NO. COUNTY OF KNOTT

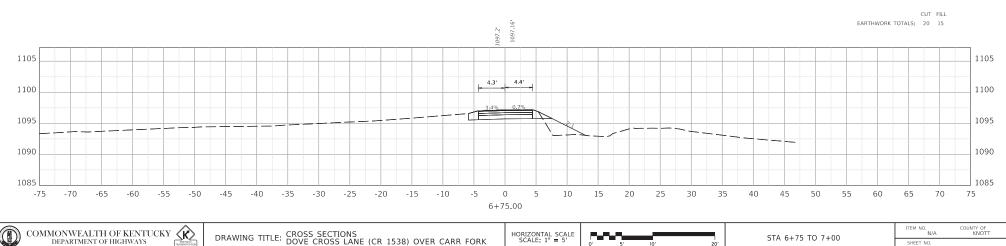
SHEET NO. R3











COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

SHEET NO.

TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS KNOTT COUNTY DOVE CROSS LANE (CR 1538) OVER CARR FORK STA. 6+27.75

ESTIMATE OF QUANTITIES																						
BID ITEM CODE	08100	08104	08151	08019	08051	08039	08033	03299	08663	08003	23378EC											
BID ITEM	Concrete Class "A"	Concrete Class "AA"	Steel Reinforcement, Epoxy Coated	Cyclopean Stone RIp-Rap	Piles - Steel HP 14 x 89	Pre-Drilling for Piles	Test Piles	Armored Edge for Concrete	Precast PC Box Beam CB21-48	Foundation Preparation	Concrete Sealing											
UNIT	C.Y.	C.Y.	LBS.	TON	L.F.	L.F.	L.F.	L.F.	L.F.	L.S.	S.F.											
End Bent #1	24.9		2136	42	40.2	39.3	25				308											
End Bent #2	31.9		2453	53	59.6	51.5	25				366											
18																						
ž																						
12																						
19		_	1											_	-							
N																						
Cuparatruatura		13.3	1558	_				24	168		1152		_	-	1		_	-	-			
Superstructure		13.3	1558					24	168		1152				-							
BRIDGE TOTALS	56.8	13.3	6147	95	99.8	90.8	50	24	168	1	1826											

NOTE: Per the General Notes, Structure Granular Backfill and Geotextile Fabric Class 2 shall be incidental to Foundation Preparation. Geotextile Fabric Class 1 shall be incidental to Cytopean Stome Rip-ray.

REPLACEMENT OF EXISTING BRIDGE NO. 060C001

DEPARTMENT OBJECT CODE: D23A SYP FOR UTILITIES: 12-0262.OTH FEMA CODE: 4663-DR

Plans Prepared By J.M. CRAWFORD & ASSOC.

SPECIFICATIONS

INDEX OF SHEETS

SPECIAL NOTES

SPECIAL PROVISIONS

STANDARD DRAWINGS BDP-001-06 Box Beam General Notes & Reference BDP-002-03 | Box Beam Bearing Details BDP-003-03 Box Beam Miscellaneous Details BDP-004-04 Box Beam Tension Rod Details BGX-006-10 Stencils for Structures BGX-022 | Joint Waterproofing BJE-001-14 Armored Edges BPS-011-04 HP14x89 Steel PIIe

RGX-100-07 Treatment of Embankments at End Bents RGX-105-09 Treatment of Embankments at End Bents-Detalls RPM-100-11 Curb and Gutter Curbs and Valley Gutter

69 Embankment at Bridge End Bent Stru-

Description

Sheet No.

S1 Title Sheet S2 General Notes S3 Layout S4 Foundation Layout End Bent #1

Concrete Sealing

End Bent #2 S7 Superstructure

Construction Elevations

9th Edition AASHTO LRFD Bridge Design Specifications (2020)

Construction

Lee A. Carlisle

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

JMC Crawford & ASSOCIATES

DESIGNED BY: Lee Caribble Consulting Engineers

DETAILED BY: Greg Crank

DATE: 9/30/2022 CHECKED BY DESIGNED BY: Lee Carlisle tuart McIntosh Lee Carlisle

TITLE SHEET CARR FORK

KNOTT CR 1538 28541

2019 Standard Specifications for Road and Bridge

SPECIFICATIONS

REFERENCES TO THE SPECIFICATIONS ARE TO THE CURRENT EDITION OF THE KENTLICKY DEPARTMENT OF HIGHWAYS STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION INCLUDING ANY CURRENT SUPPLEMENTAL SPECIFICATIONS. ALL REFERENCES TO THE AASHTO SPECIFICATIONS ARE TO THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 9TH EDITION WITH INTERIMS

DESIGN LOAD

THIS BRIDGE IS DESIGNED FOR KYHL-93 LIVE LOAD, (I.E. 1.25XAASHTO HL93 LIVE LOAD). THIS BRIDGE IS DESIGNED FOR A FUTURE WEARING SURFACE OF 15 PSF

DESIGN METHOD

ALL REINFORCED CONCRETE MEMBERS ARE DESIGNED TO BE EQUIVALENT OR GREATER THAN THE LOAD AND RESISTANCE FACTOR DESIGN METHOD AS SPECIFIED IN THE CURRENT AASHTO SPECIFICATIONS.

MATERIALS DESIGN SPECIFICATIONS

FOR CLASS "A" REINFORCED CONCRETE F'C = 3500 PSI FOR CLASS "AA" REINFORCED CONCRETE F'C = 4000 PSI FOR STEEL REINFORCEMENT FY = 60000 PSI

MATERIAL SPECIFICATIONS

AASHTO M153

AASHTO SPECIFICATIONS OR ASTM. CURRENT EDITION, AS DESIGNATED BELOW SHALL GOVERN THE MATERIALS FURNISHED

PREFORMED CORK EXPANSION JOINT MATERIAL

AASHTO M-31 DEFORMED AND PLAIN BILLET-STEEL FOR CONCRETE REINFORCEMENT,

PREMOUDED CORK FILLER TYPE II

PREFORMED CORK EXPANSION JOINT MATERIAL SHALL CONFORM TO SUBSECTION 807.04.02 (TYPE II) OF THE KENTLICKY DEPARTMENT OF HIGHWAYS STANDARD SPECIFICATIONS.

CLASS "AA" CONCRETE IS TO BE USED THROUGHOUT THE SUPERSTRUCTURE CLASS "A" CONCRETE IS TO BE USED THROUGHOUT THE SUBSTRUCTURE, PRESTRESSED BEAM CONCRETE SHALL BE IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS.

REINFORCEMENT

DIMENSIONS SHOWN FROM THE FACE OF CONCRETE TO BARS ARE TO CENTER OF BARS LINESS OTHERWISE SHOWN, SPACING OF BARS IS FROM CENTER TO CENTER OF BARS, ANY REINFORCING BARS DESIGNATED BY SUFFIX "E" IN THE PLANS SHALL BE EPOXY COATED IN ACCORDANCE WITH SECTION 811.10 OF THE STANDARD SPECIFICATIONS. ANY REINFORCING BARS DESIGNATED BY SUFFIX "S" IN A BILL OF REINFORCEMENT SHALL BE CONSIDERED A STIRRUP FOR PURPOSES OF BEND DIAMETERS.

CONSTRUCTION IDENTIFICATION

THE NAMES OF THE PRIME CONTRACTOR AND THE SUB-CONTRACTOR SHALL BE IMPRINTED IN THE CONCRETE WITH 1 INCH LETTERS AT A LOCATION DESIGNATED BY THE ENGINEER. THE CONTRACTOR SHALL FURNISH ALL PLANS, EQUIPMENT AND LABOR NECESSARY TO DO THE WORK FOR WHICH NO DIRECT PAYMENT WILL BE MADE

BEVELED EDGES

ALL EXPOSED EDGES SHALL BE BEVELED 3/4", UNLESS OTHERWISE SHOWN.

PAYMENT FOR PRECAST CONCRETE BEAMS

THE BASIS OF PAYMENT FOR THE PRESTRESSED CONCRETE BEAMS SHALL BE AT THE CONTRACT UNIT PRICE PER LINEAR FOOT OF BEAM, IN ACCORDANCE WITH THE SPECIFICATIONS,

SLOPE PROTECTION

SLOPE PROTECTION AT END BENTS SHALL BE DRY CYCLOPEAN STONE RIPRAP MEETING THE REQUIREMENTS OF SECTIONS 703 AND 805 OF THE SPECIFICATIONS. GEOTEXTILE FABRIC, CLASS 1 SHALL BE PLACED BETWEEN THE EMBANKMENT AND THE SLOPE PROTECTION IN ACCORDANCE WITH STANDARD SPECIFICATIONS 214 AND 843. PAYMENT FOR GEOTEXTILE FABRIC, CLASS 1, SHALL BE CONSIDERED INCIDENTAL TO THE UNIT PRICE BID FOR DRY CYCLOPEAN STONE RIPRAP.

COMPLETION OF THE STRUCTURE

THE CONTRACTOR IS REQUIRED TO COMPLETE THE STRUCTURE IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. MATERIAL, LABOR, OR CONSTRUCTION OPERATIONS NOT OTHERWISE SPECIFIED, ARE TO BE INCLUDED IN THE BID ITEM MOST APPROPRIATE TO THE WORK INVOLVED. THIS MAY INCLUDE COFFERDAMS, SHORING, EXCAVATIONS, BACKFILLING, REMOVAL OF ALL OR PARTS OF EXISTING STRUCTURES, PHASE CONSTRUCTION INCIDENTAL MATERIALS LARGE OR ANYTHING FLSE REQUIRED TO COMPLETE THE

SHOP DRAWING PROCEDURE

- 1. FABRICATORS SHALL SUBMIT ALL REQUIRED SHOP PLANS, BY E-MAIL, TO THE DESIGN CONSULTANT FOR REVIEW.
- , DESIGNERS WILL MAKE REVIEW COMMENTS ON THESE ELECTRONIC SUBMISSIONS AS NEEDED AND RETURN THEM TO THE FABRICATOR
- 3. UPON RECONCILIATION OF THE DESIGNER'S COMMENTS, FILES SHALL BE RETURNED TO THE DESIGNER. 4. EACH SHEET WILL BE ELECTRONICALLY STAMPED BY THE DESIGNER AND PLANS WILL BE FORWARDED BY EMAIL TO THE KYTC DIVISION OF STRUCTURAL DESIGN'S SHOP PLAN COORDINATOR FOR DISTRIBUTION. EMBIL TO THE ATTENDED DIRECTLY TO THE SHOP PLAN COORDINATOR WILL BE DISTRIBUTED AND ONLY PLANS SUBMITTED DIRECTLY TO THE SHOP PLAN COORDINATOR WILL BE DISTRIBUTED AND ONLY PLANS CONTAINING BOTH THE "DISTRIBUTED BY THE DIVISION OF STRUCTURAL DESIGN" AND THE DESIGNER REVIEW STAMP ARE TO BE USED FOR FABRICATION.

UTILITIES

THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING ANY AND ALL EXISTING UTILITIES PRIOR TO EXCAVATION OF MATERIAL OR INSTALLATION OF GUARDRAIL OR OTHER CONSTRUCTION ACTIVITIES THAT MAY INVOLVE UTILITIES (OVERHEAD OR UNDERGROUND)

VERIFYING FIELD CONDITIONS

THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS BEFORE ORDERING MATERIAL, NEW MATERIAL THAT IS UNSUITABLE BECAUSE OF VARIATIONS IN THE EXISTING STRUCTURE SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE

DIMENSIONS

DIMENSIONS ARE FOR A NORMAL TEMPERATURE OF 60 DEGREES FAHRENHEIT, LAYOUT DIMENSIONS ARE HORIZONTAL DIMENSIONS.

SUPERSTRUCTURE SLAB

THE SUPERSTRUCTURE SLAB SHALL BE POURED CONTINUOUSLY FROM END TO END OF SLAB BEFORE THE CONCRETE

MASTIC TAPE

APPLY MASTIC TAPE AT BRIDGE IN ACCORDANCE WITH STANDARD DRAWING BGX-022, C.E. THE CONTRACTOR SHALL FURNISH ALL PLANS, EQUIPMENT, AND LABOR NECESSARY TO DO THE WORK FOR WHICH NO DIRECT PAYMENT WILL BE MADE.

TEMPORARY SUPPORTS

TEMPORARY SUPPORTS OR SHORING WILL NOT BE PERMITTED UNDER THE BEAMS WHEN POURING THE CONCRETE DECK SLAB OR WHEN TAKING "TOP OF BEAM" ELEVATIONS.

ARMORED EDGE

FARRICATE ARMORED EDGE TO MATCH CROSS SLOPE AND PARABOLIC CROWN AT EACH END OF BRIDGE

FOUNDATION PREPARATION

FOUNDATION PREPARATION SHALL BE IN ACCORDANCE WITH SECTION 603 OF THE SPECIFICATIONS.

FOUNDATION EXCAVATIONS SHOULD BE PROPERLY BRACED/SHORED TO PROVIDE ADEQUATE SAFETY TO PERSONS WORKING IN OR AROUND EXCAVATIONS, BRACING SHOULD BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL STATE AND LOCAL GUIDELINES.

TEMPORARY SHORING, SHEETING, COFFERDAMS, AND/OR DEWATERING METHODS MAY BE REQUIRED TO FACILITATE FOUNDATION CONSTRUCTION, IT SHOULD BE ANTICIPATED THAT GROUNDWATER WILL BE ENCOUNTERED AT FOUNDATION LOCATIONS WITHIN THE FLOOD PLAIN.

TEMPORARY SHORING, BRACING, SHEETING, COFFERDAMS AND DEWATERING SHALL BE INCLUDED IN THE LUMP SUM BID FOR FOUNDATION PREPARATION.

STRUCTURE GRANULAR BACKFILL

THE ESTIMATED QUANTITY OF STRUCTURE GRANULAR BACKELL IS 123.7 C.Y. AND MATERIALS FOR STRUCTURE GRANULAR BACKFILL SHALL BE IN ACCORDANCE WITH SECTION 805 OF THE SPECIFICATIONS

CONTRARY TO THE SPECIFICATIONS, STRUCTURE GRANULAR BACKFILL WILL NOT BE MEASURED FOR PAYMENT BUT SHALL BE INCLUDED IN THE LUMP SUM BID FOR FOUNDATION PREPARATION.

CONCRETE SEALER

APPLY CONCRETE SEALER IN ACCORDANCE WITH THE SPECIAL NOTE FOR CONCRETE SEALING. SEE SHEET S7 FOR LIMITS OF SEALING ON THE SUPERSTRUCTURE.

GEOTEXTILE FABRIC

GEOTEXTILE FABRIC SHALL CONFORM TO THE STANDARD SPECIFICATIONS

GEOTEXTILE FABRIC CLASS 1 IS TO BE PLACED BETWEEN THE EMBANKMENT AND SLOPE PROTECTION AT LOCATIONS WHERE CYCLOPEAN STONE RIP-RAP IS SPECIFIED. PAYMENT FOR FABRIC IS TO BE INCIDENTAL TO THE SLOPE PROTECTION

GEOTEXTILE FABRIC CLASS 2 IS TO BE PLACED AROUND THE STRUCTURE GRANULAR BACKELL AS SHOWN ON STANDARD DRAWING RGX-105 AND IN ACCORDANCE WITH SPECIAL PROVISION 69. PAYMENT FOR FABRIC IS TO BE INCIDENTAL TO THE LUMP SUM BID FOR FOUNDATION PREPARATION.

PRE-DRILLING FOR PILES AT END BENTS

PRE-DRILLING FOR PILES IS REQUIRED. SEE THE FOUNDATION LAYOUT. SHEET S4. FOR DETAILS.

PILE POINTS

PILE POINTS ARE NOT REQUIRED FOR THIS BRIDGE, PER THE KYTC DIVISION OF STRUCTUAL DESIGN. 2022 INTERIM GUIDANCE, PILE POINTS MAY BE LEFT OFF WHERE PILES ARE FULLY PRE-DRILLED INTO SOLID ROCK.

PILING

PILING SHALL BE PRE-DRILLED AND DRIVEN TO SATISFY THE DRIVING CRITERIA DEFINED 1N THE PILE RECORD ON SHEET S4.

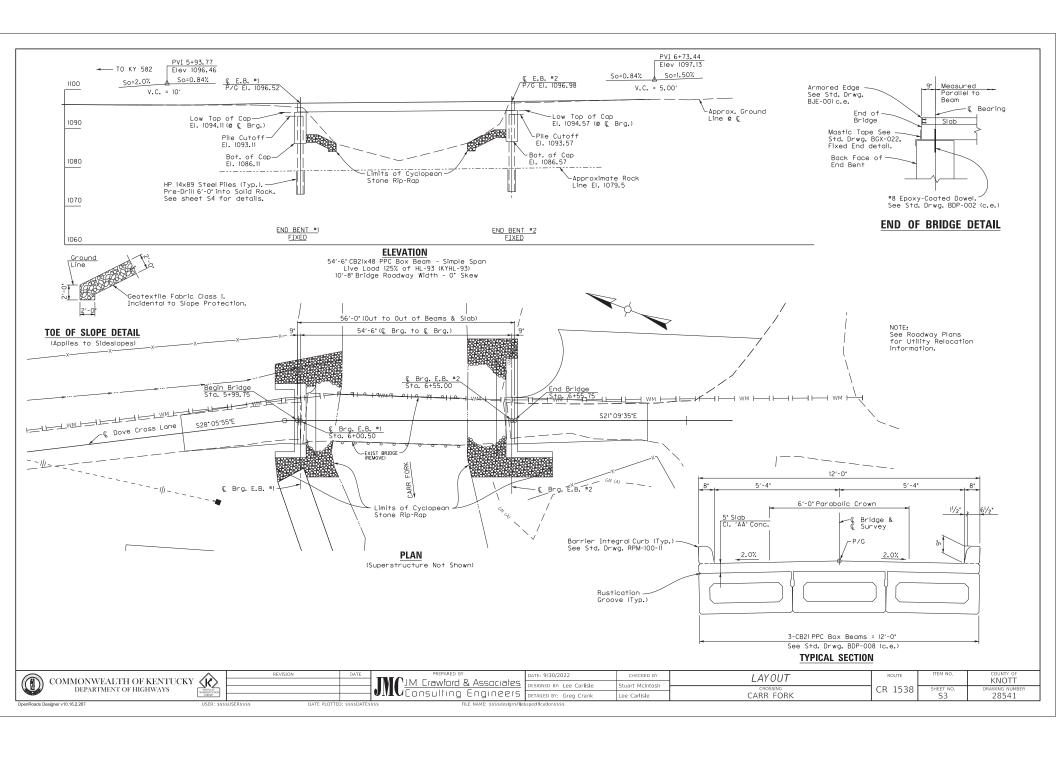
TEST PILES SHALL BE PRE-DRILLED AND DRIVEN WHERE DESIGNATED ON THE PLANS TO DETERMINE THE LENGTH OF PILE REQUIRED.

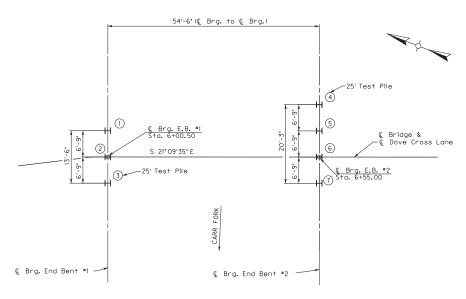
ALL TEST PILES SHALL BE ACCURATELY LOCATED SO THAT THEY MAY BE USED IN THE FINISHED

CONTRARY TO THE STANDARD DRAWINGS FOR STEEL PILING. MILL TEST REPORTS ARE NOT REQUIRED TO BE NOTARIZED.

EMBANKMENTS

CONSTRUCT THE EMBANKMENTS IN ACCORDANCE WITH SPECIAL PROVISION 69:





FOUNDATION LAYOUT

H Denotes HP14x89 Vertical Piles

PILE RECORD FOR POINT BEARING PILES												
Pile No.	Pile Cut–off Elevation	Pile Length In Place	Point of Pile Elevation As Driven	Design Axial Load								
	FEET	FEET	FEET	TONS								
END BENT #1												
- 1	1093.11			90								
2	1093.11			90								
3	1093.11			90								
END BENT #2												
4	1093.57			90								
5	1093.57			90								
6	1093.57			90								
7	1093.57			90								

Definitions of Terms

PILE CUT-OFF ELEVATION: Elevation of the top of pile in the finished structure. PILE LENGTH IN PLACE: Actual pile length below the Pile Cut-Off Elevation in the finished

POINT OF PILE ELEVATION AS DRIVEN: Actual point of pile elevation in the finished

DESIGN AXIAL LOAD: Load carried by each pile as estimated from structural design calculations for Factored LRFD Loadings.

CALCULATED FIELD BEARING: Controry to Section 604.03.07 of the Standard Specifications, in place bearing values are not required for piles bearing on rock when driven to practical refusal.

Driving Criteria

DRIVING CRITERIA: Drive point bearing piles to practical refusal.

PRACTICAL REFUSAL (Case 2): For this project minimum blow requirements are reached PRACTICAL REFUSAL (Case 2½ for this project minimum blow requirements are reached after total penetration becomes 1/2 inch or less for 10 consecutive blows, practical refusal is obtained after the pile is struck on additional 10 blows with total penetration of 1/2 inch or less, Advance the production piling to the driving resistances specified above and to depths determined by test pilets) and subsurface data sheetisl, Immediately cease driving operations if the pile visibly yields or becomes damaged during driving. If hard driving is encountered because of dense strata or an obstruction, such as a boulder before the pile is advanced to the depth anticipated, the Engineer will determine if more blows than the overage driving resistance specified for practical refusal is required to further advance the pile. Drive additional production and test piles if directed by the Engineer.

Field Data

For each pile, the Project Engineer shall record the following on this sheet: Pile Length in Place and Point of Pile Elevation as Driven.

Submit this record to:

Kentucky Transportation Cabinet Director, Division of Structural Design 3rd Floor East 200 Mero Street Frankfort, KY 40622

This pile record does not replace other pile records the Project Engineer is required to keep and submit.

Use HP 14x89 in accordance with BPS-011, c.e.

Notes

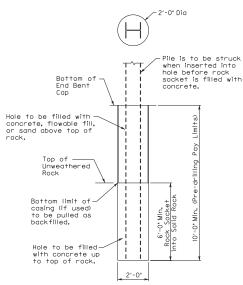
HAMMER CRITERIA: A hammer with a rated energy of between 20 and 30 kip-ft will be required to drive the H-piles to practical refusal without encountering excessive blow counts or damaging the pile. The contractor shall submit the proposed pile driving system to the Department for approval prior to the installation of the first pile, approval of the pile driving system by the Engineer will be subject to satisfactory field performance of the pile driving procedures.

PRE-DRILLING PILES: Pre-drilling will be required at End Bents 1& 2. Use 24-inch diameter holes with a minimum embedment of 6 feet into sound bedrock and a minimum of 10 feet total embedment below the cap. The pile must be struck with a pile hammer once in place to ensure that adequate capacity and refusal has been achieved. The rock socket shall then be filled with Class B concrete conforming to Section 601 of the Standard Specifications; however, provide a mix with a 6 to 10 linch slump at the time of placement, High range water reducing and retarding admixtures and Class F flyash may be used to obtain this slump. Cosing or some other method of maintaining an open hole above the rock socket may be needed for installation of the piles and concrete. If casing is used, it must be removed, as the hole above the rock socket is backfilled with concrete, flowable fill, or sand. Care must be taken that the piling is located correctly since the piling is an integral part of the structure and protrudes up into the cap. The cost of all materials, labor, and equipment required to pre-drill, drive piles to refusal, place concrete, and backfill the holes shall be included in the price per linear foot for Pre-Drilling Piles.

Orient Piles as shown in the Foundation Plan.

Cofferdams and/or dewatering methods may be required to facilitate foundation construction of pile caps.

Temporary sheeting and/or shoring may be required for installation of pile caps. The contractor shall be responsible for the stability and safety of all excavations



PRE-DRILLING DETAIL

NOTE: Maintain 6'-0" Min. rock socket into solid unweathered bedrock.

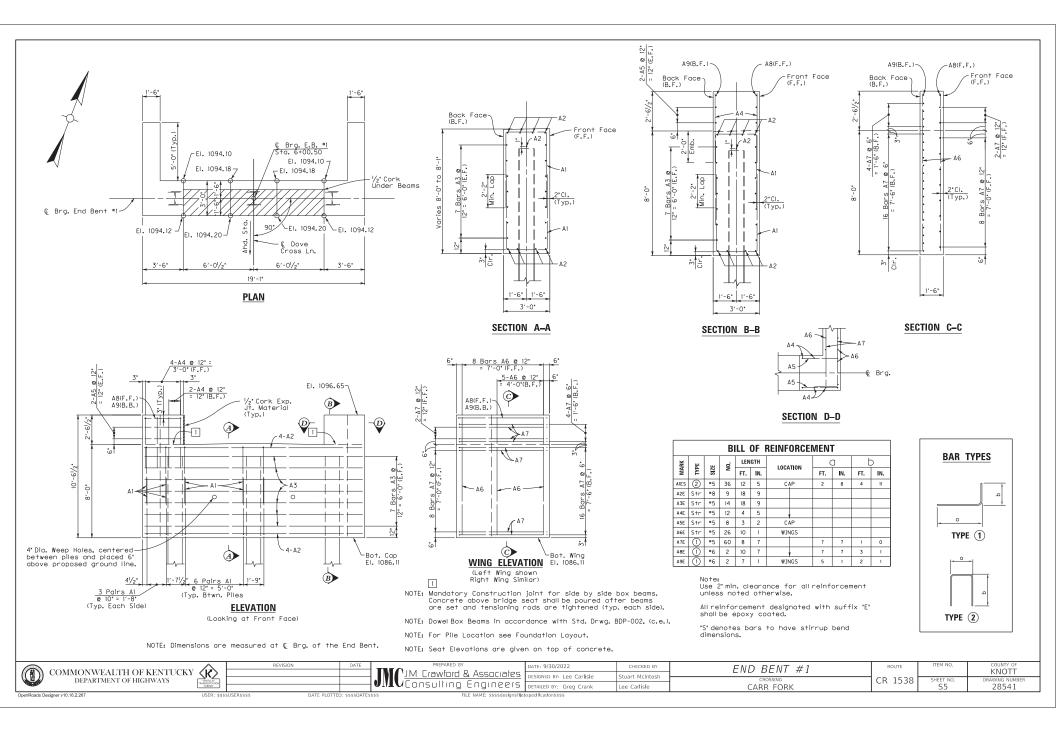
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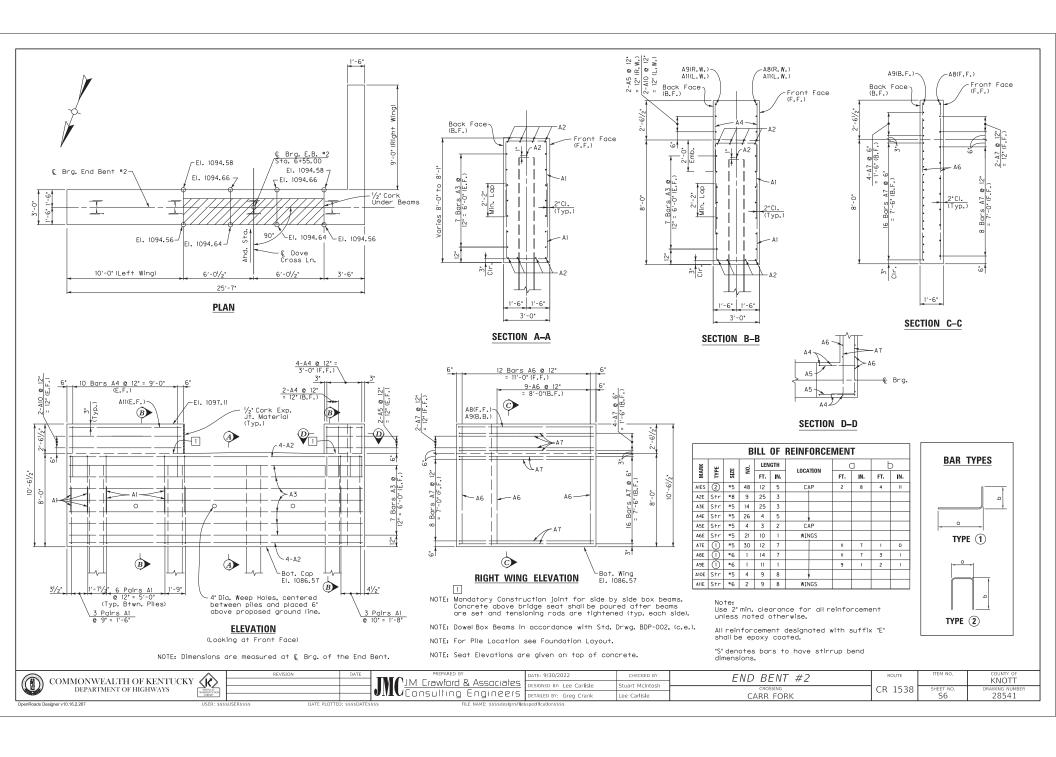
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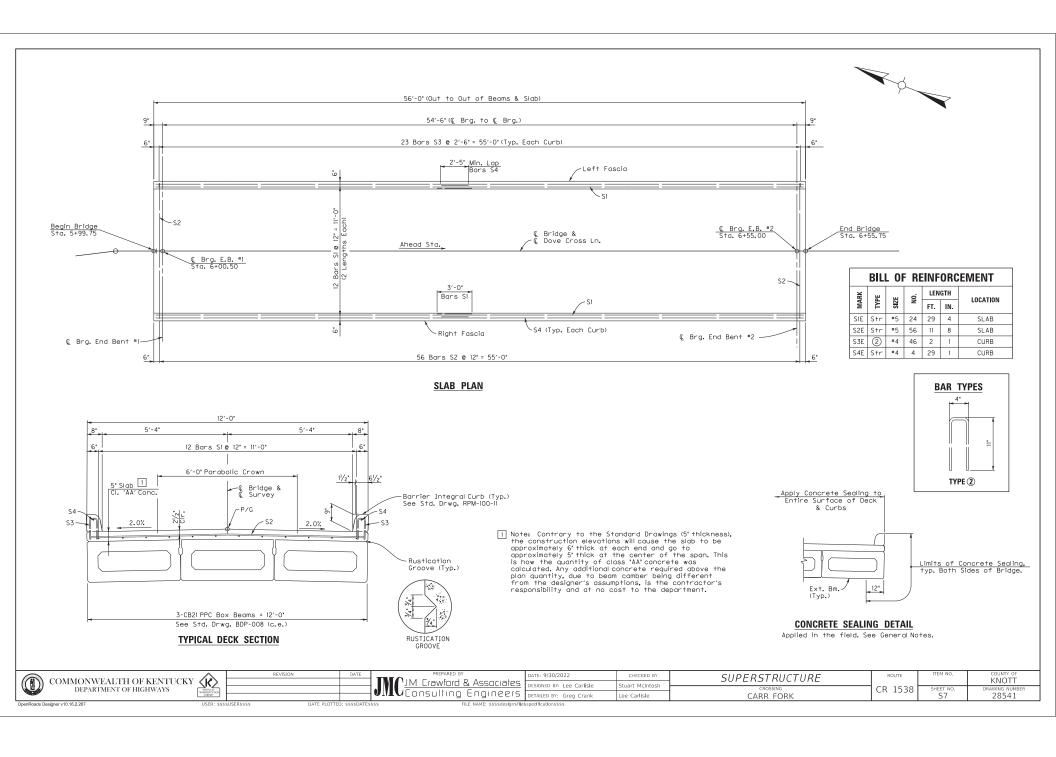


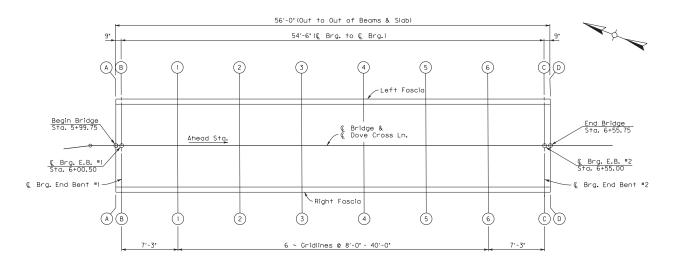
 Γ J.M. Crawford & Associates JMC Consulting Engineers DATE: 9/30/2022 CHECKED BY DESIGNED BY: Lee Carlisle tuart McIntosh Lee Carlisle

FOUNDATION LAYOUT ROUTE CR 1538 SHEET N CARR FORK

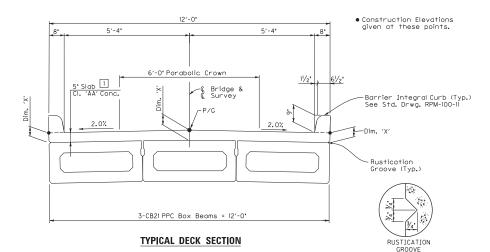


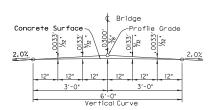






GRID LAYOUT





PARABOLIC CROWN DETAIL

NOTES FOR ELEVATIONS TAKEN ON PRESTRESSED CONCRETE BEAMS

Take elevations on top of beam at points indicated after the beams have been laterally tensioned and grouted. The beam elevations are to be read to three decimal places and entered in tables under "Top of Beam" elevations.

Compute dimension 'X' as follows: 'Construction Elevation' minus
'Top of Beam' elevation equals dimension 'X'. Construction Elevations
include camber due to weight of the concrete slab and barrier. Measuring
of dimension 'X' gives the final check on beam tolerances for camber,
beam damage, and errors in erection that produce reverse cambers,
sags, and unsightly fascia beams,
The minimum allowable dimension 'X' or slab thickness is 4 3/4'(0, 395').
The maximum allowable dimension 'X' or slab thickness is 6'(0,500').
If any computed dimension 'X' is outside limits, adjustments need to be
made to the dimensions 'X' on one or more gridlines at the discretion of
the Engineer.

made to the the Engineer.

For setting templates, measure dimension "X" above top of beams for top of template. Do not set template by elevations.

Temporary supports or shoring will not be permitted under the girders when pouring the concrete floor slab or when taking "Top of Beam" elevations.

Note: The Table of Elevations at Centerline includes the 3/8" deduction for Parabolic Crown.

See Note Ion sheet S7.

CONSTRUCTION ELEVATIONS												
	l I	LEFT FASCIA	١		FILE GRAD		RIGHT FASCIA					
LOCATION	CONSTR. ELEV.	TOP OF BEAM	"X"	CONSTR. ELEV.	TOP OF BEAM	"X"	CONSTR. ELEV.	TOP OF BEAM	DIM.			
SKEW LINE A-A	1096.390			1096.480			1096.390					
SKEW LINE B-B	1096.397			1096.487			1096.397					
SKEW LINE C-C	1096.857			1096.947			1096.857					
SKEW LINE D-D	1096.863			1096.953			1096.863					
GRID LINE 1	1096.467			1096.557			1096.467					
GRID LINE 2	1096.544			1096.634			1096.544					
GRID LINE 3	1096.621			1096.711			1096.621					
GRID LINE 4	1096.689			1096.779			1096.689					
GRID LINE 5	1096.747			1096.837			1096.747					
GRID LINE 6	1096.804			1096.894			1096.804					

CO CO	MMONWEALTH OF KENTUCKY 🗸	k E	REVISION DAT	TE T	PREPARED BY TMCJM Crawford & Associates 5	DATE: 9/30/2022	CHECKED BY	CONSTRUCTION ELEVATIONS	ROUTE CR 1538	ITEM NO.	COUNTY OF KNOTT
	DEPARTMENT OF HIGHWAYS	HTUCKY				DESIGNED BT. LEE CATTISTE	Stuart McIntosn	CROSSING		SHEET NO.	DRAWING NUMBER
-	[AERRET			JIIIVConsulting Engineers 7	DETAILED BY: Greg Crank	Lee Carlisle	CARR FORK		S8	28541
penRoads Designer v	v10.16.2.267 U	SER: \$\$\$\$U	ISERSSSS DATE PLOTTED: SSSSE	DATESSSS	FILE NAME: SSSSdeslonSflleSsc	pecificationssss					